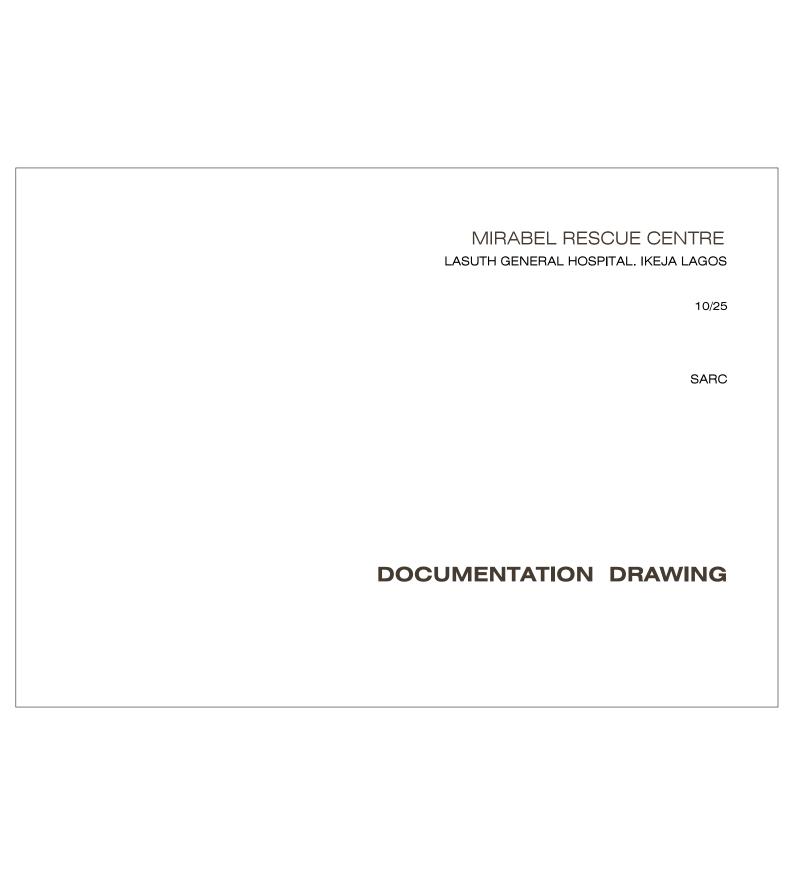
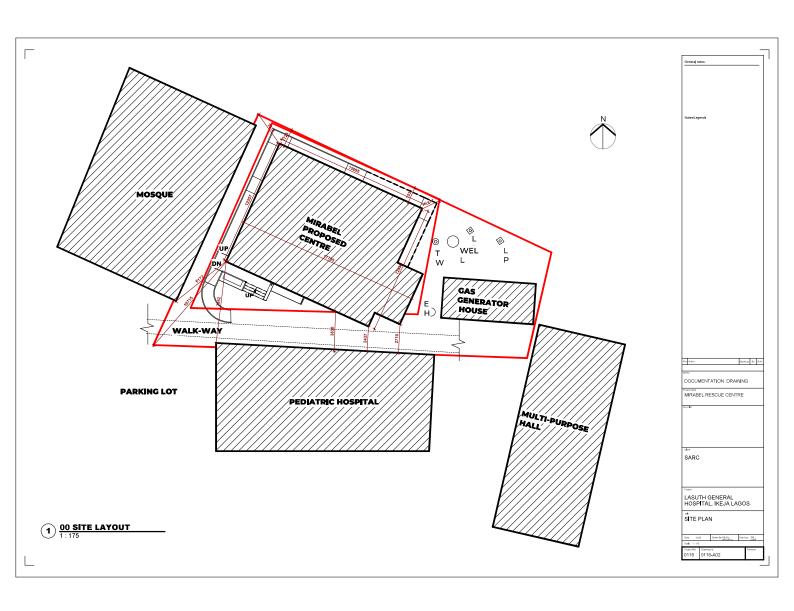
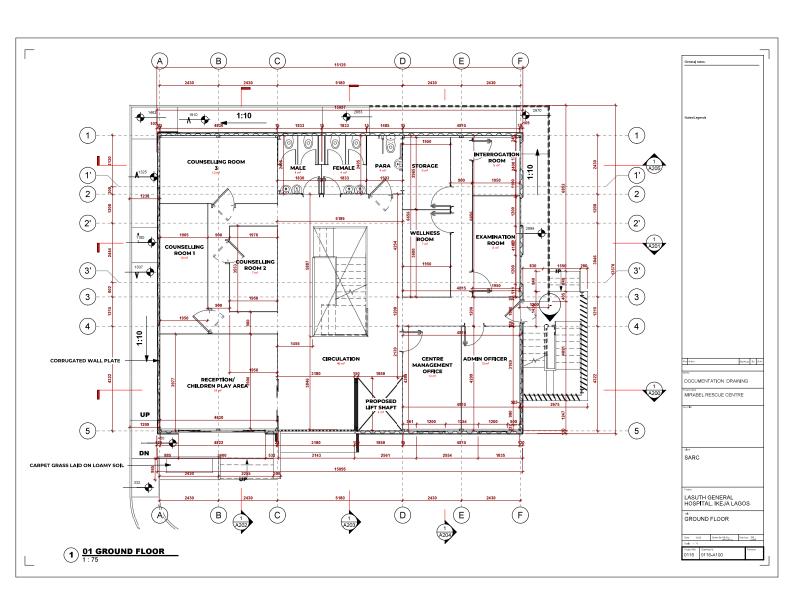
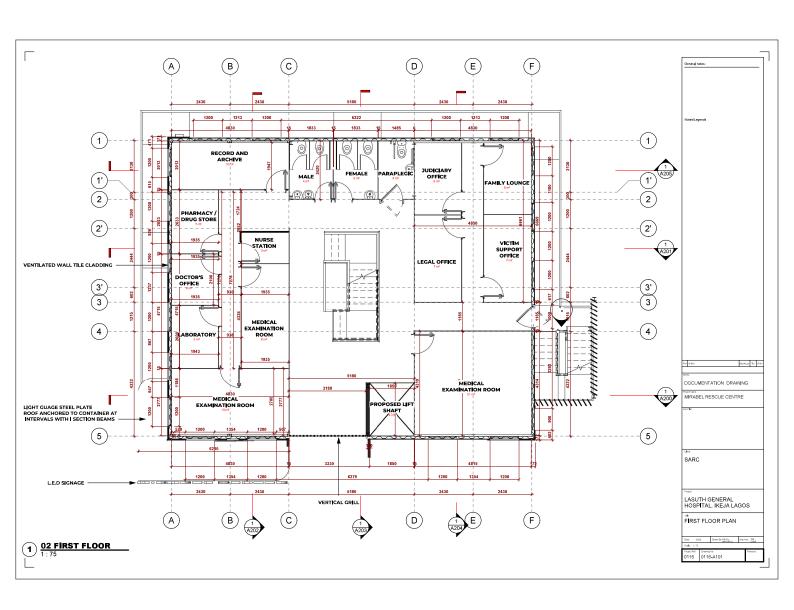
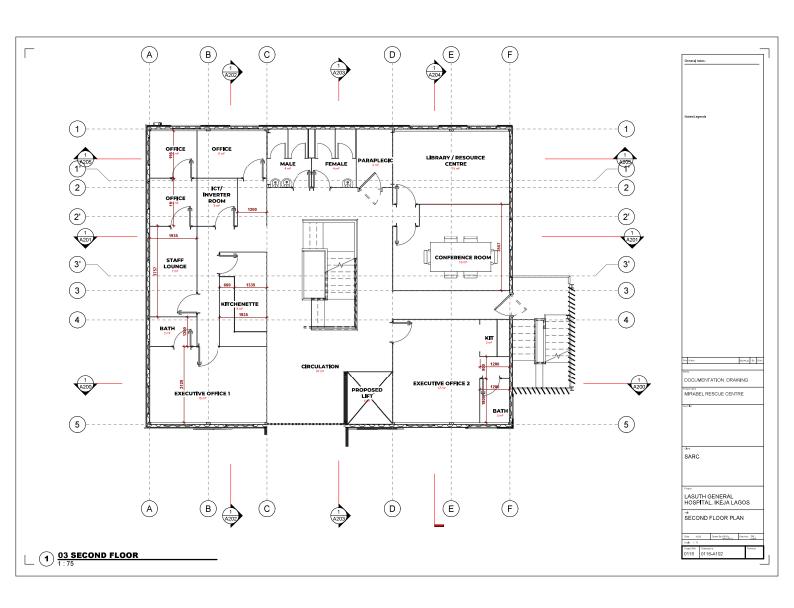
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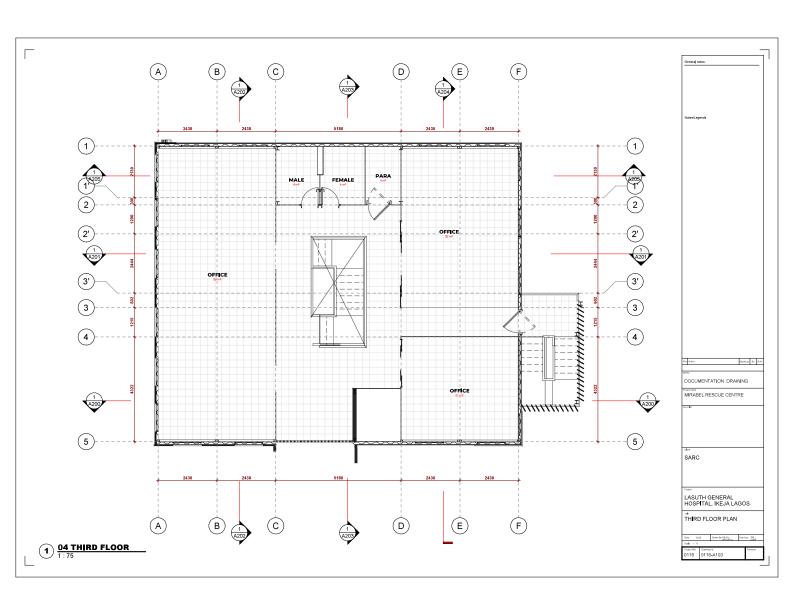


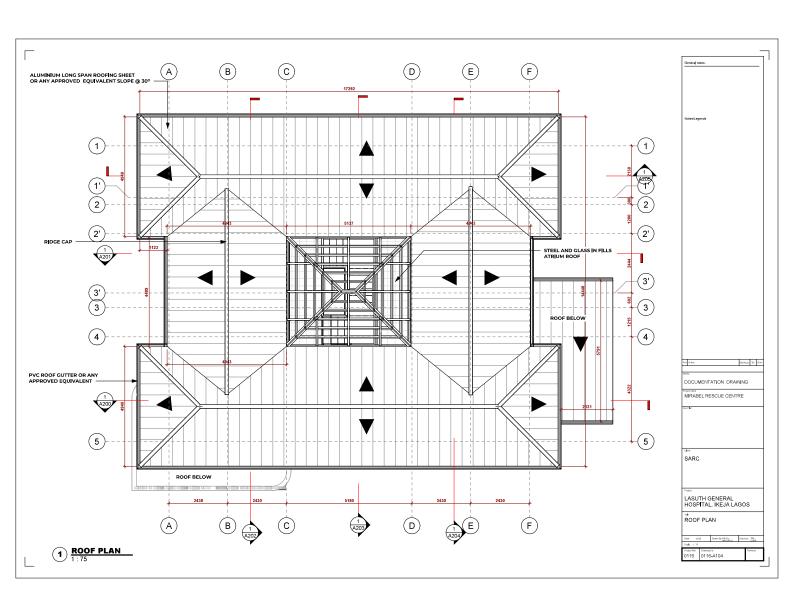


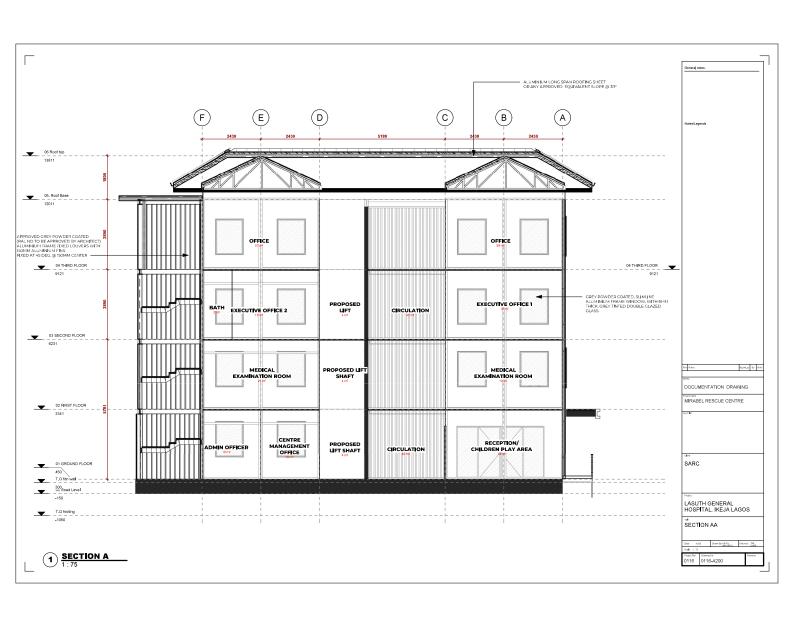


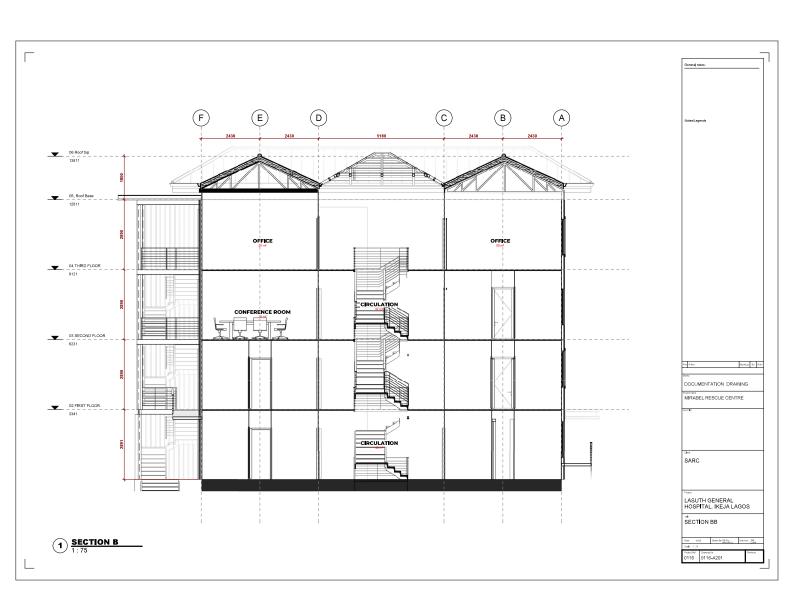


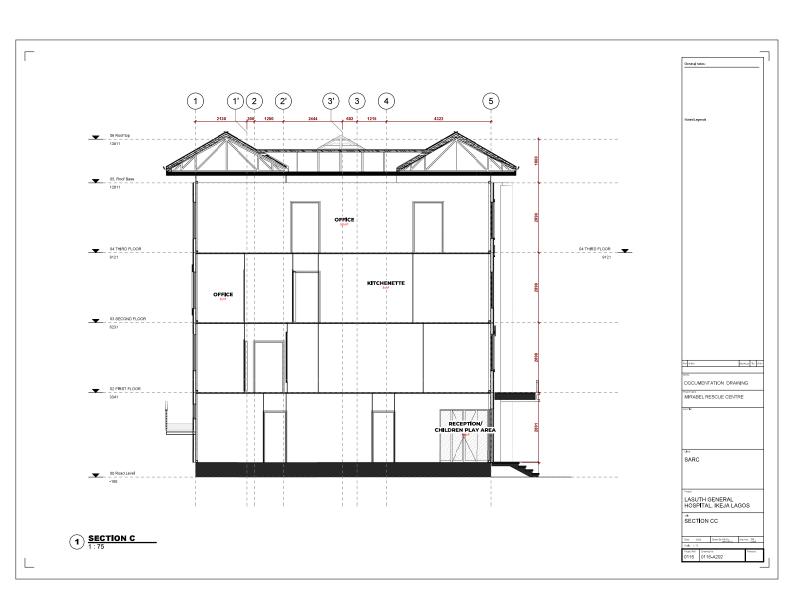








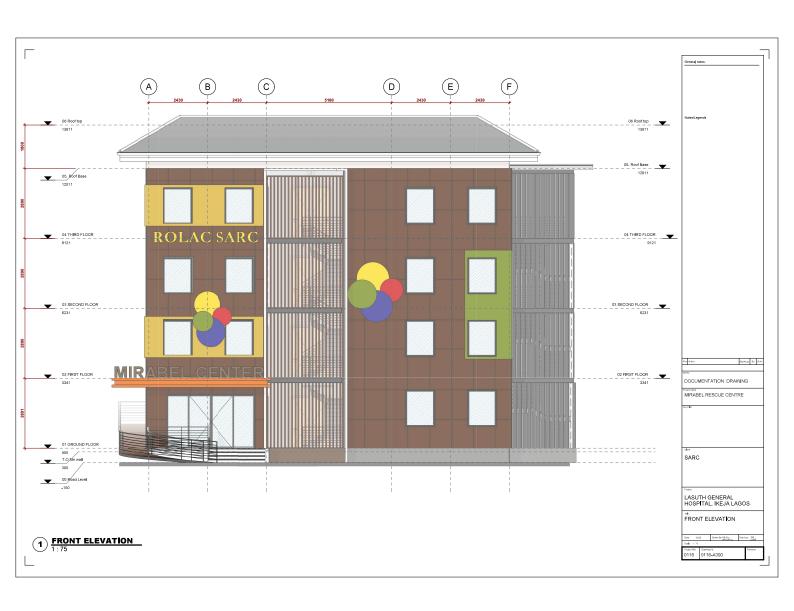




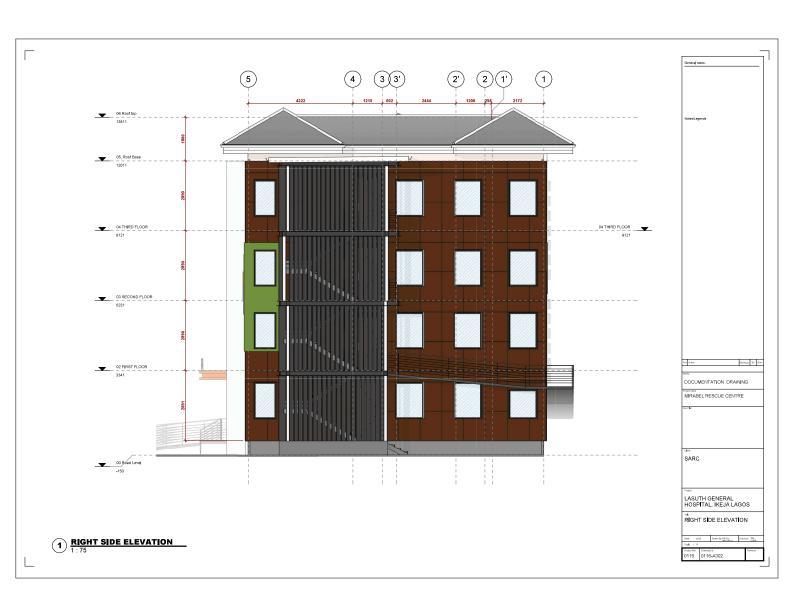


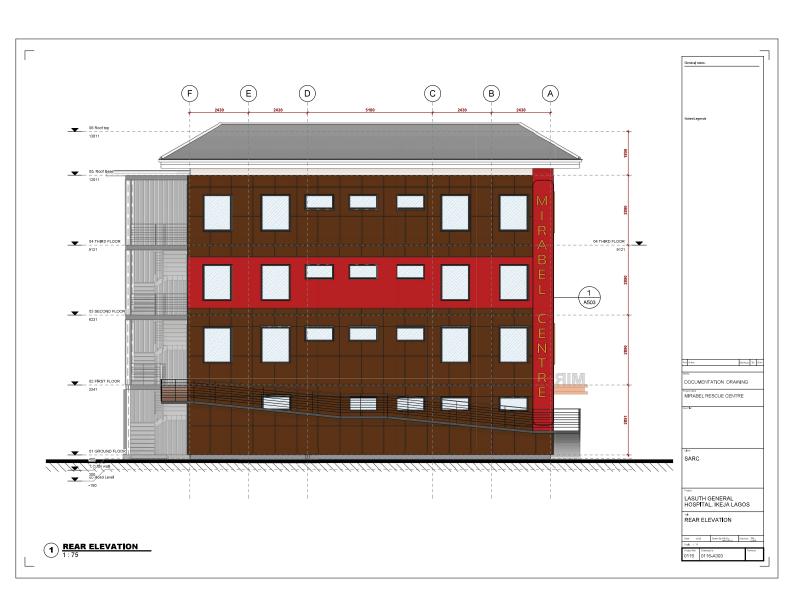


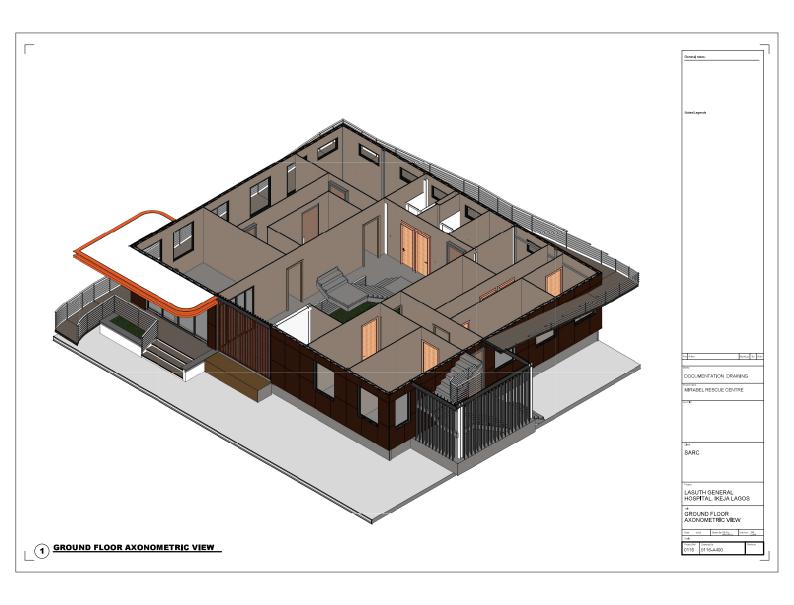


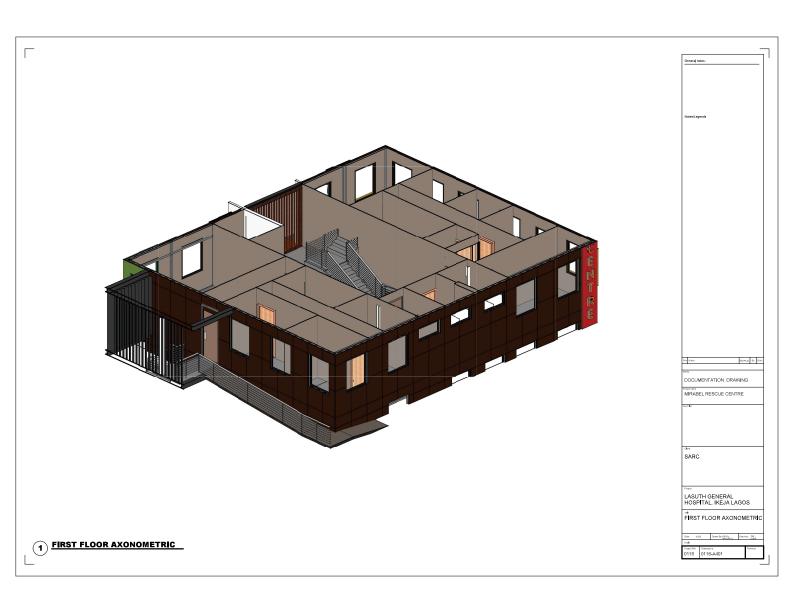


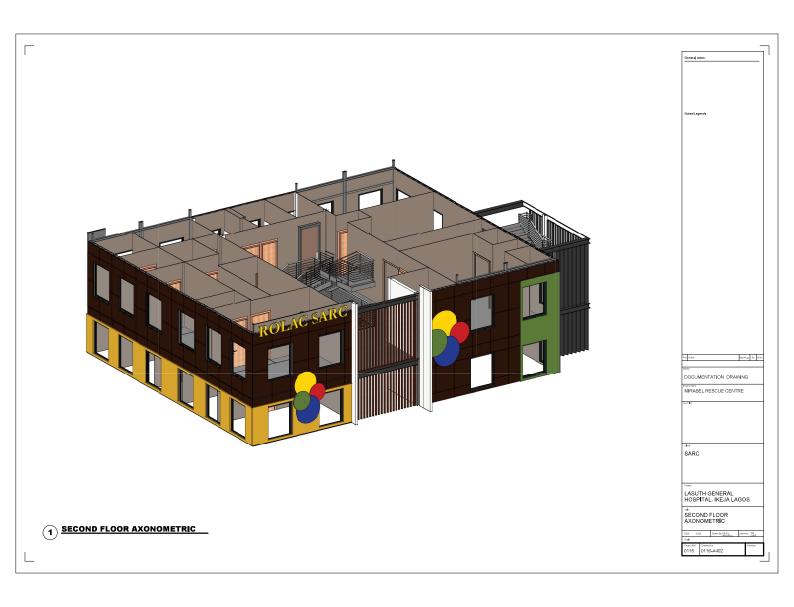


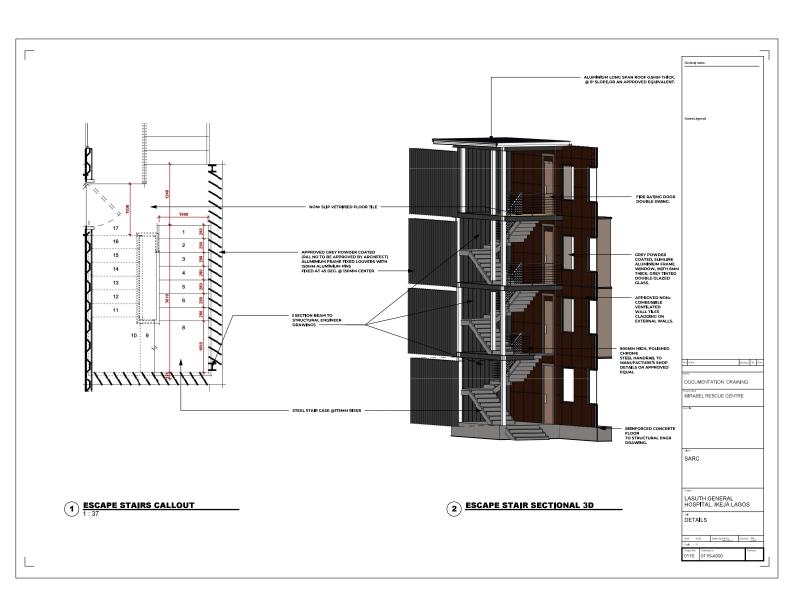


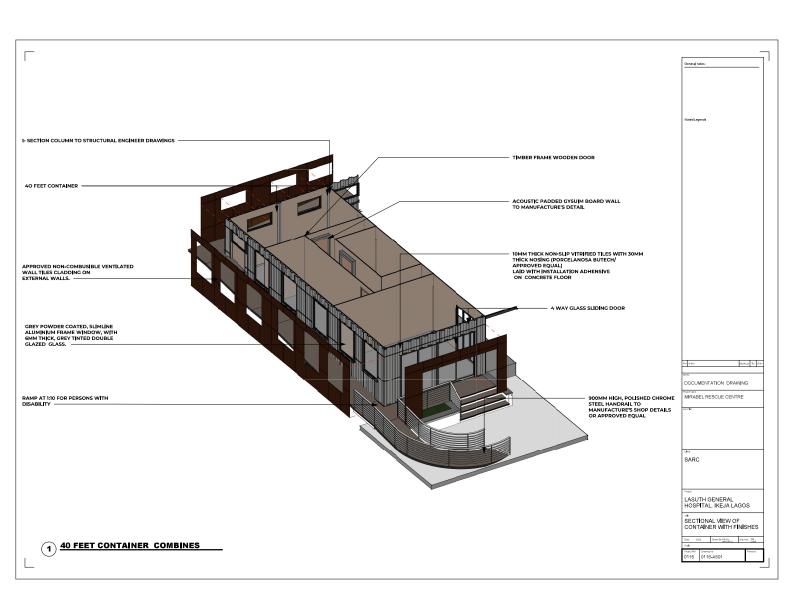


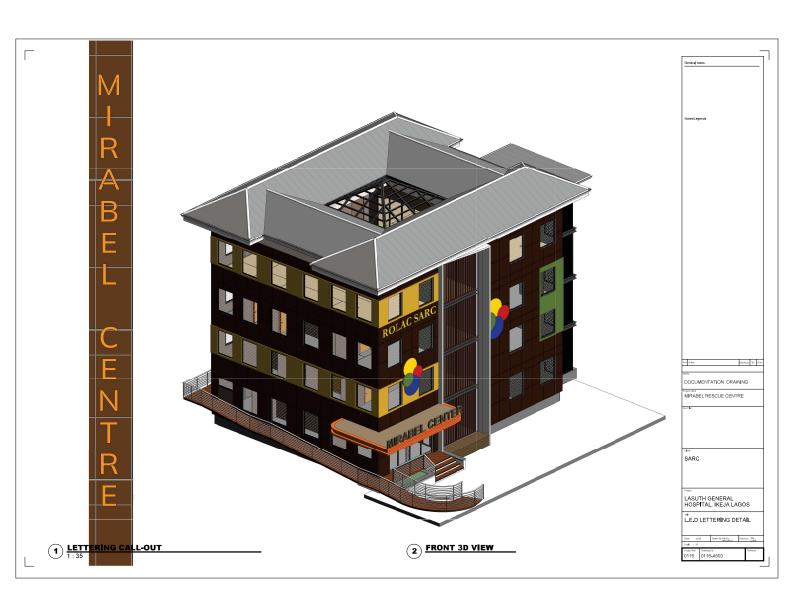


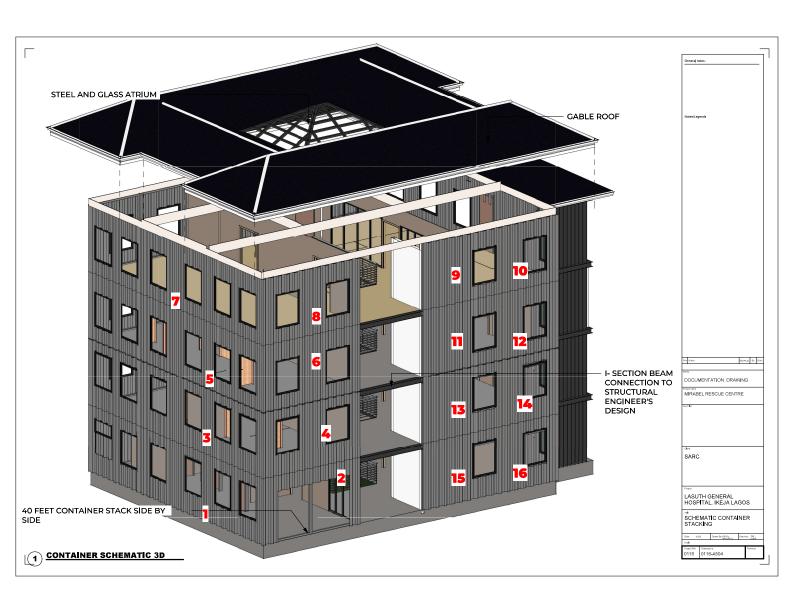




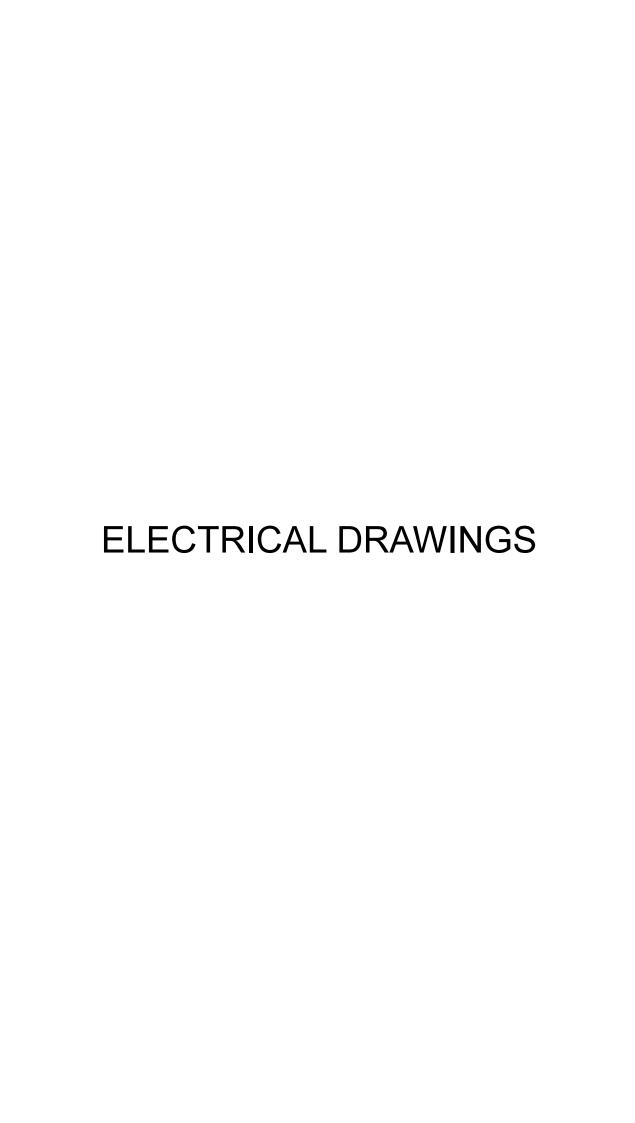








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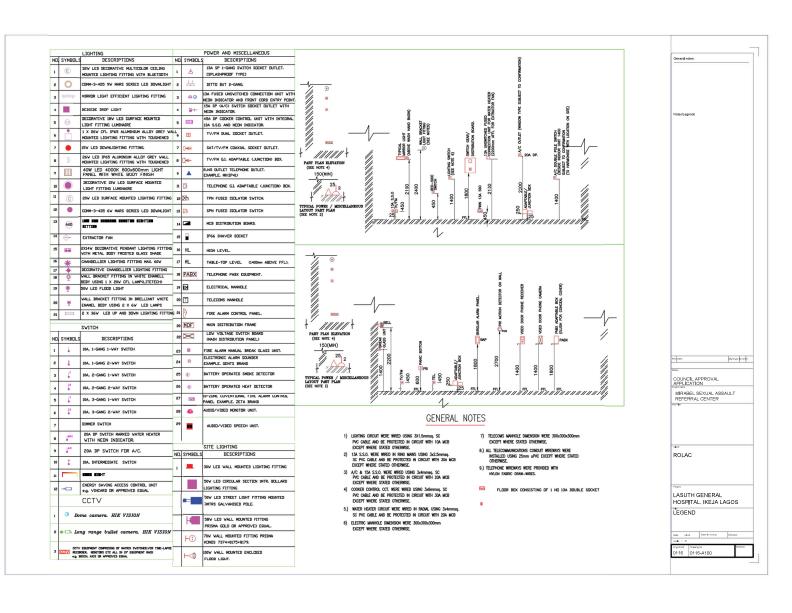
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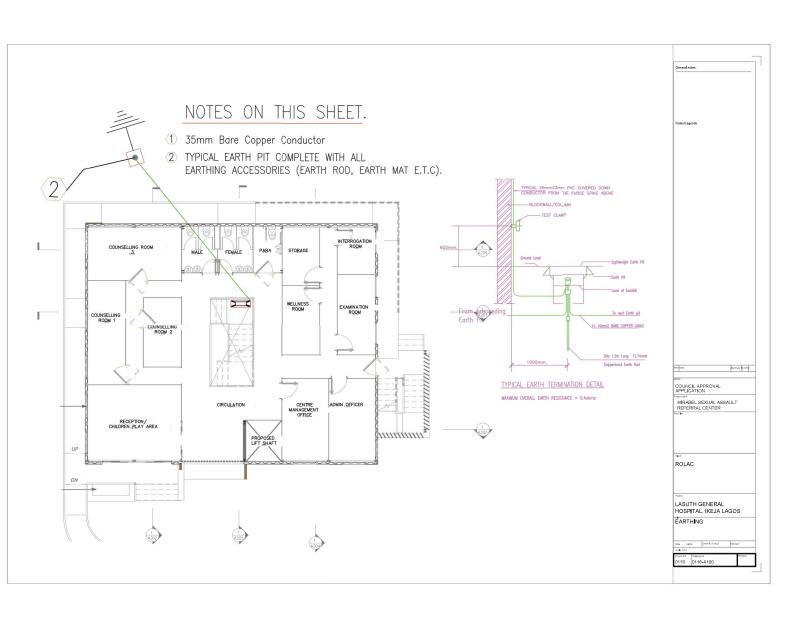
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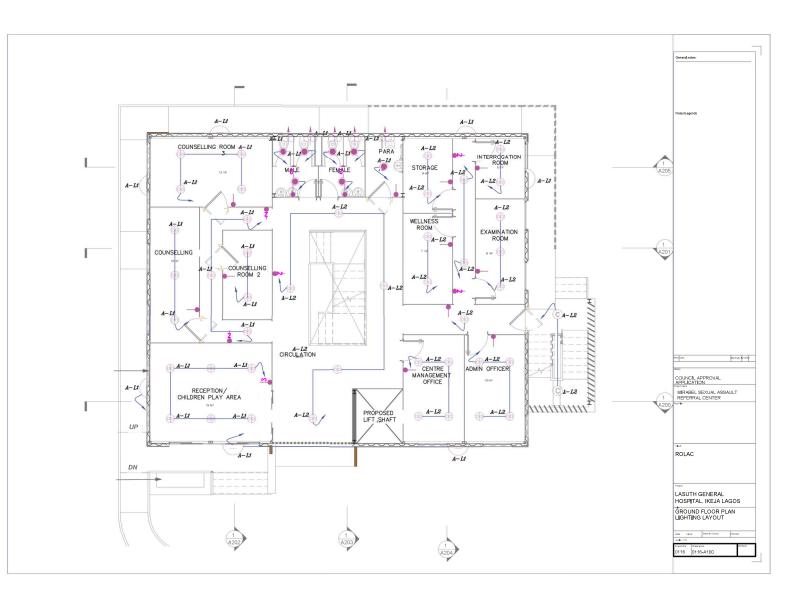
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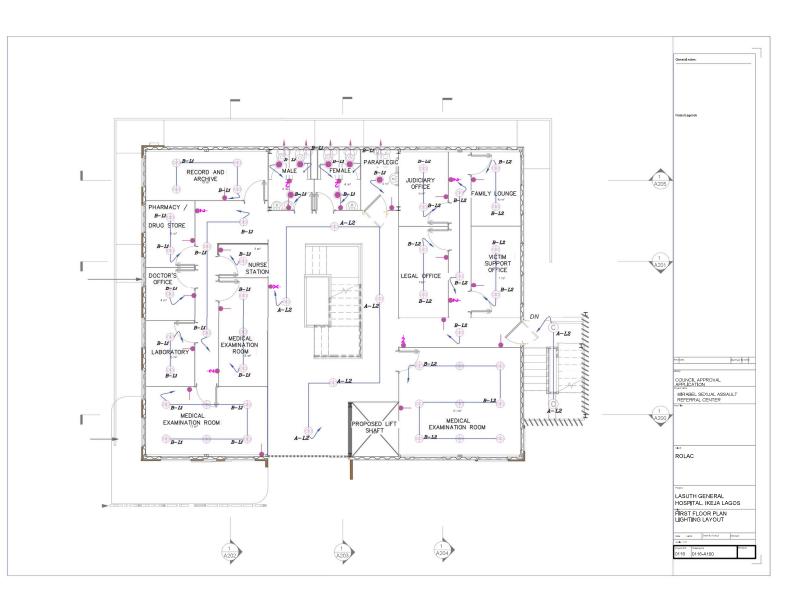
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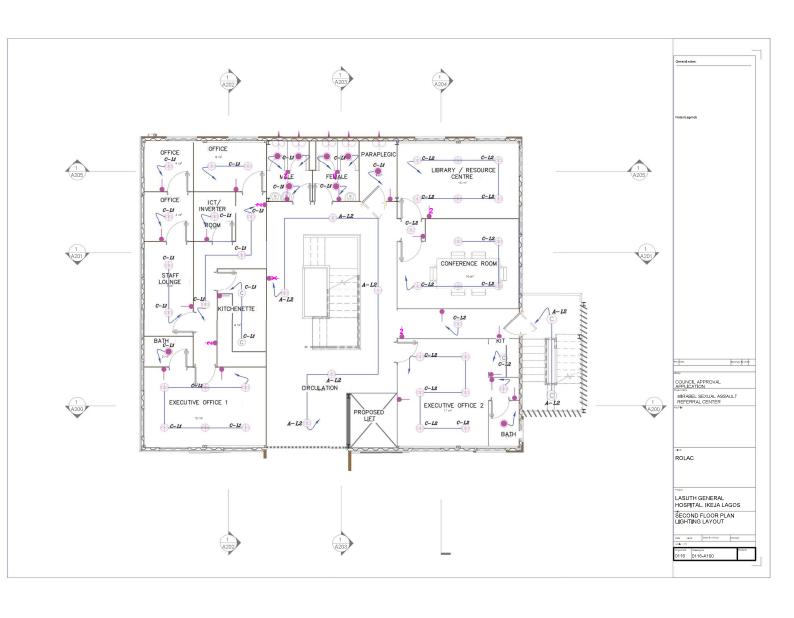
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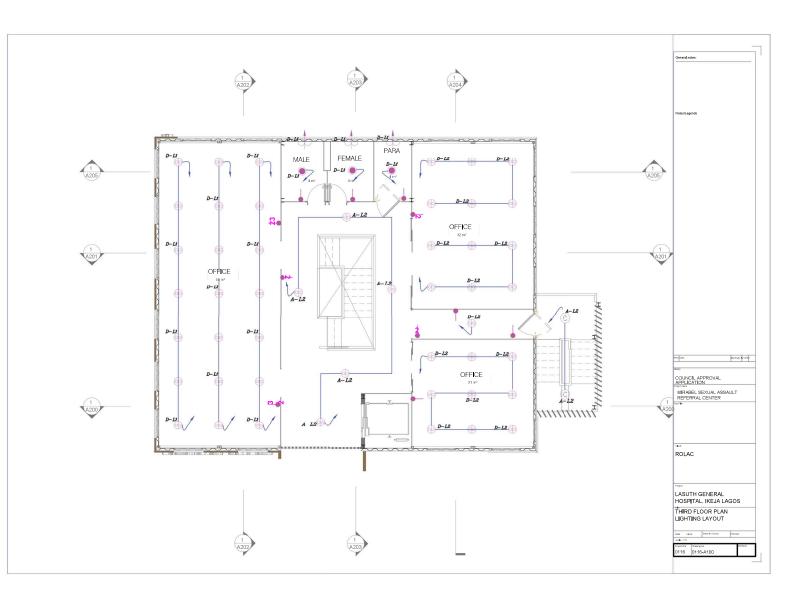




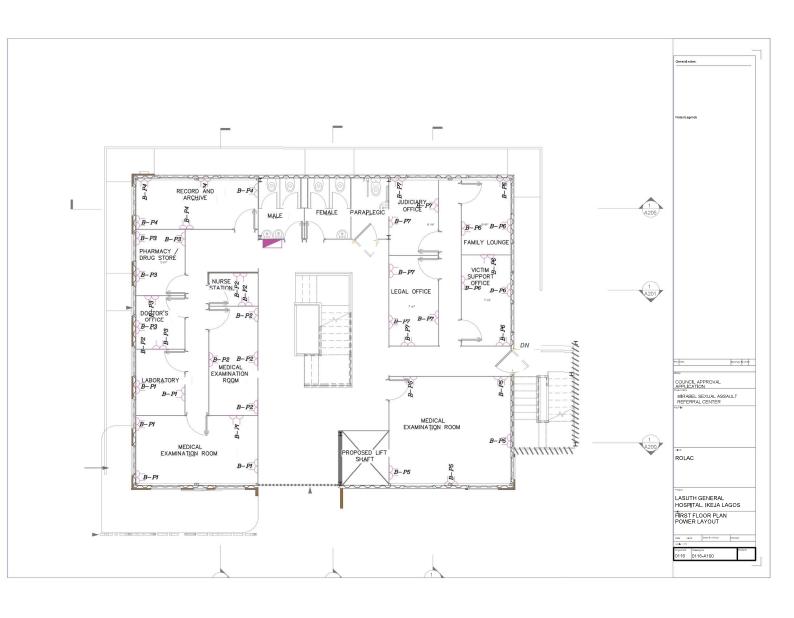




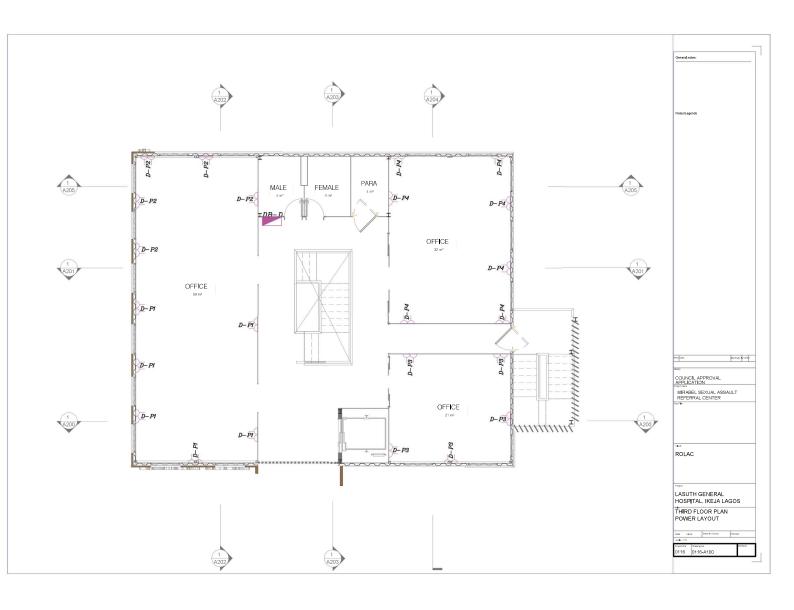


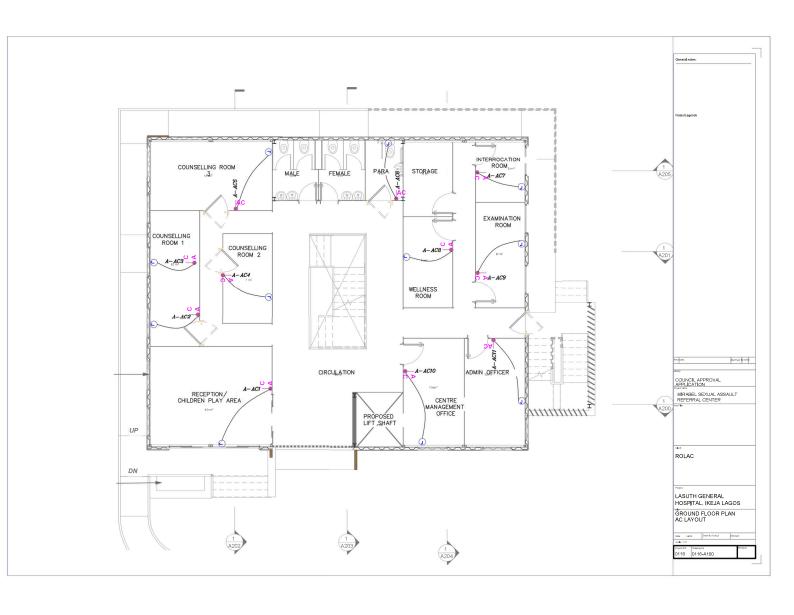


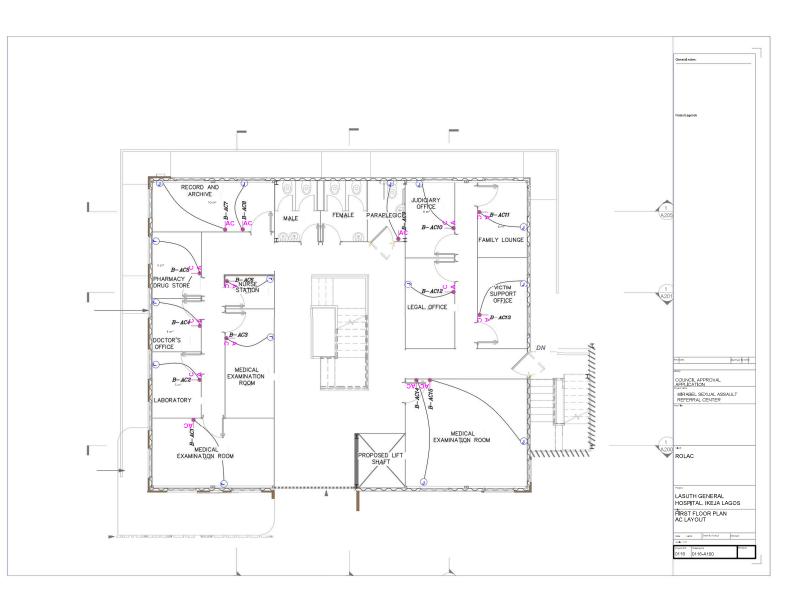




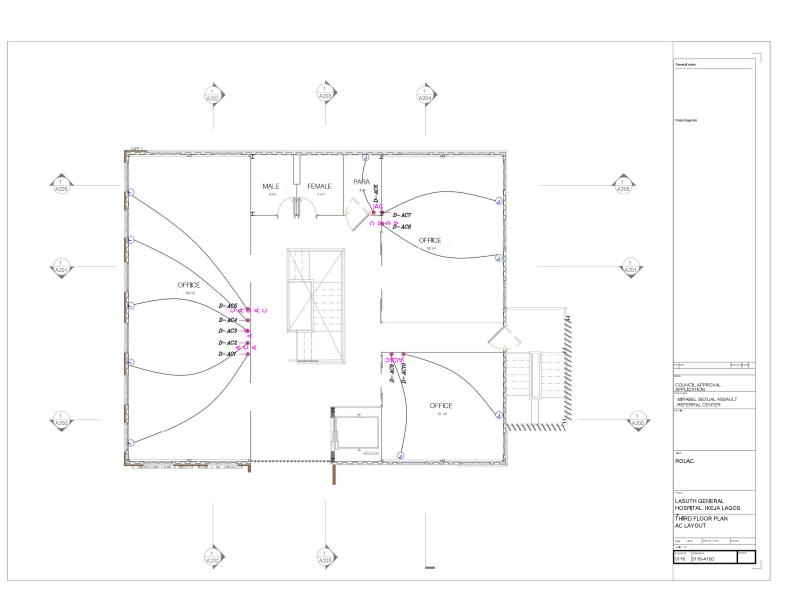


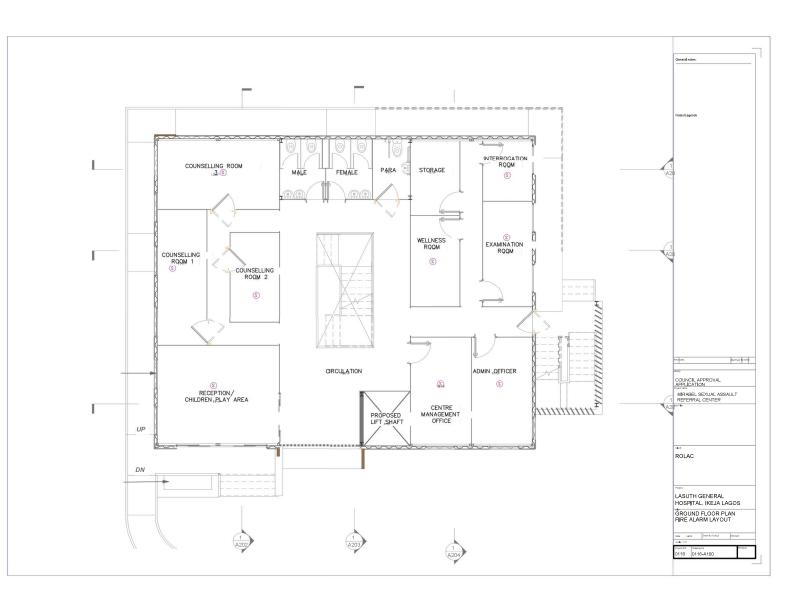


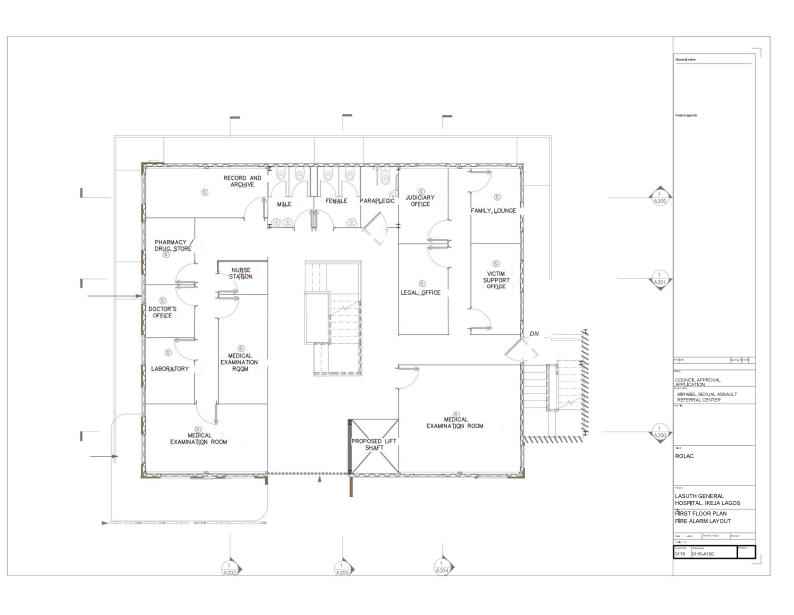




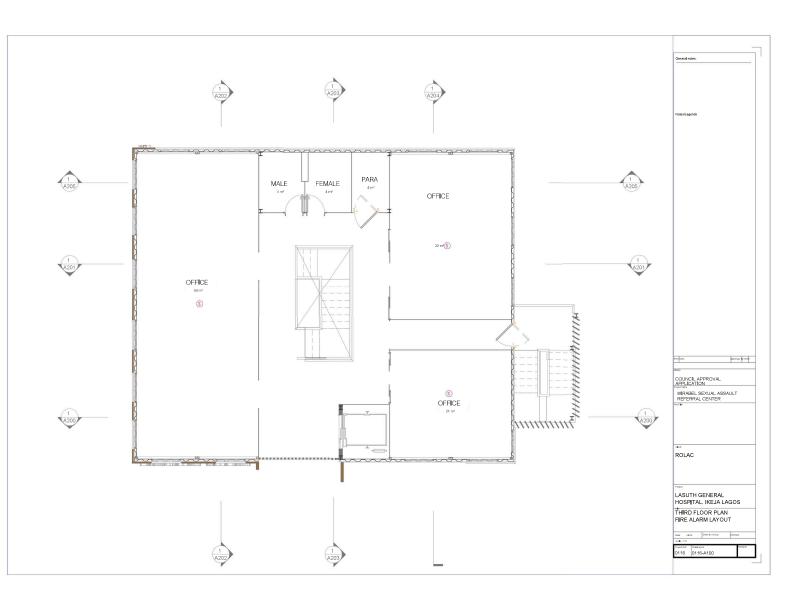


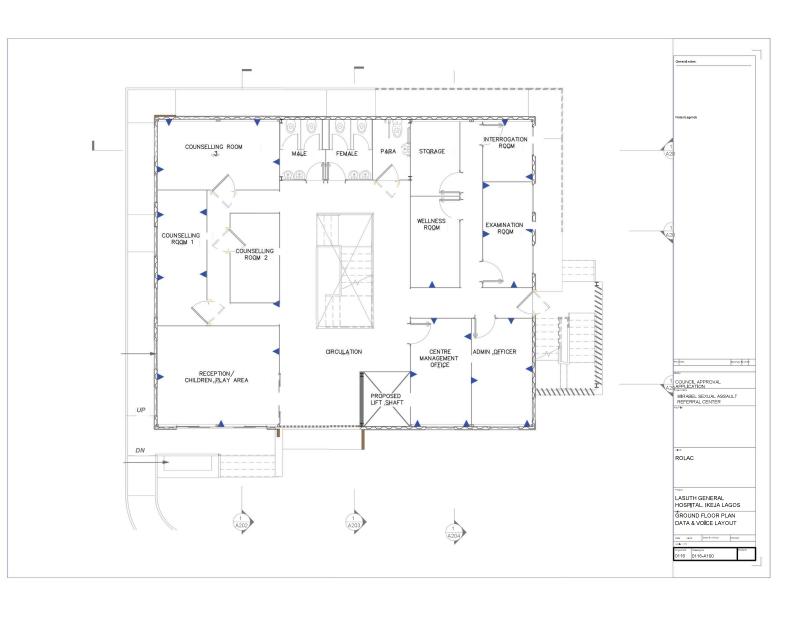


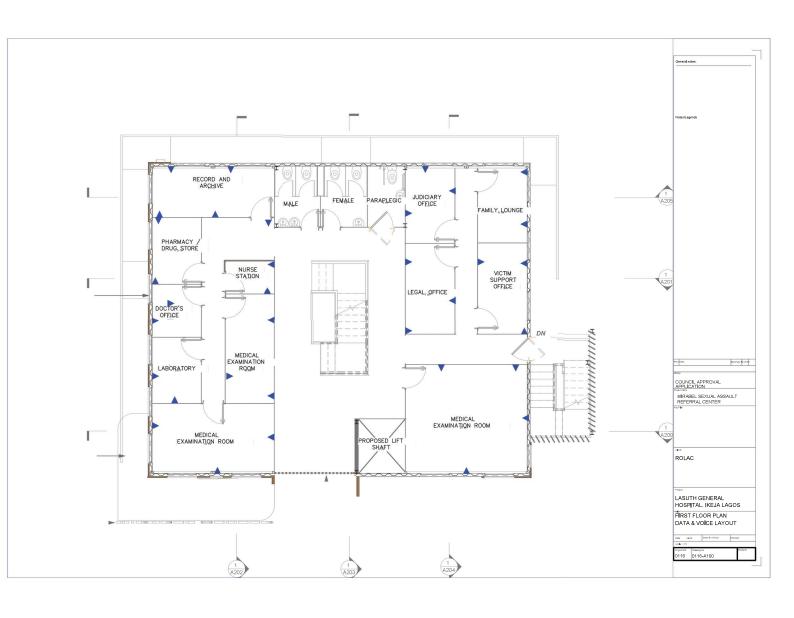


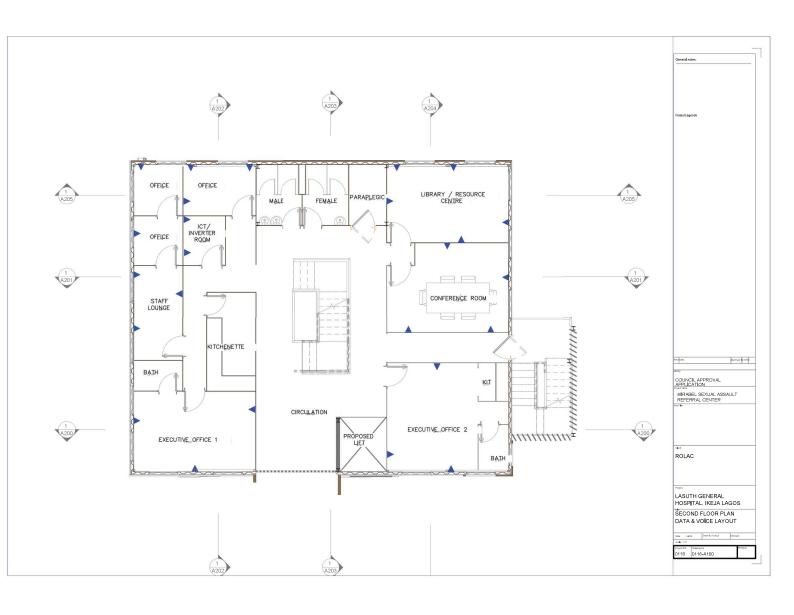


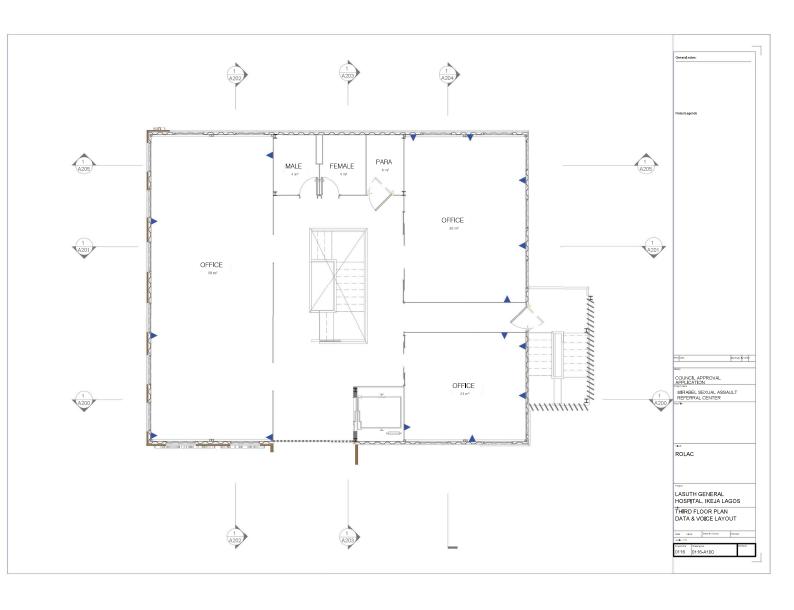


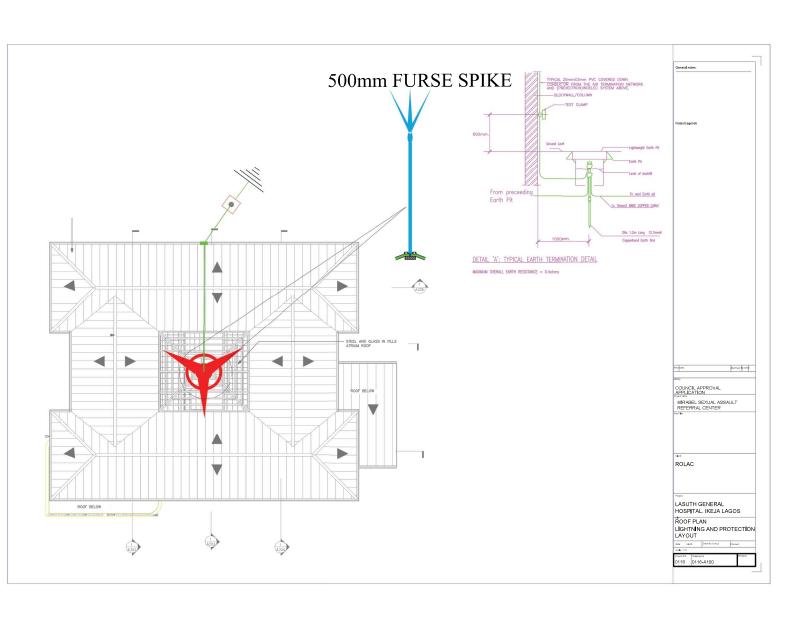






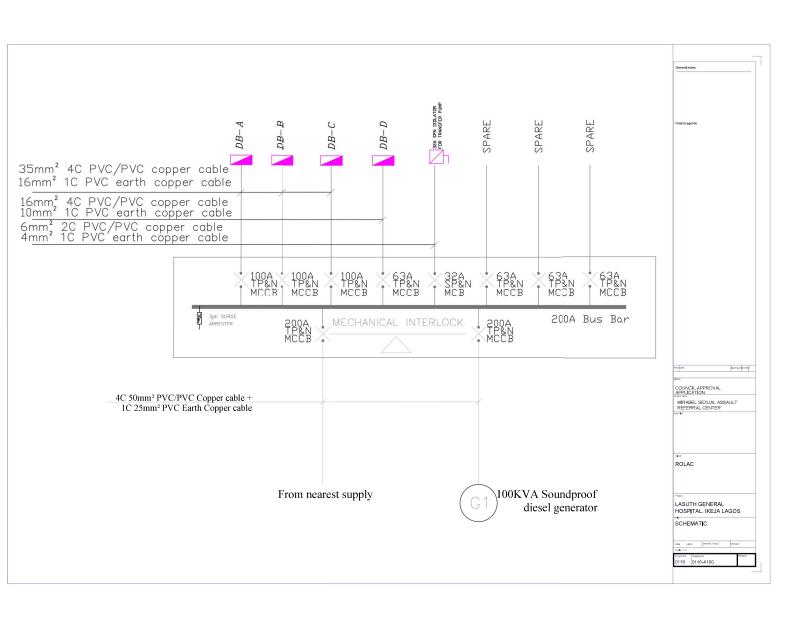






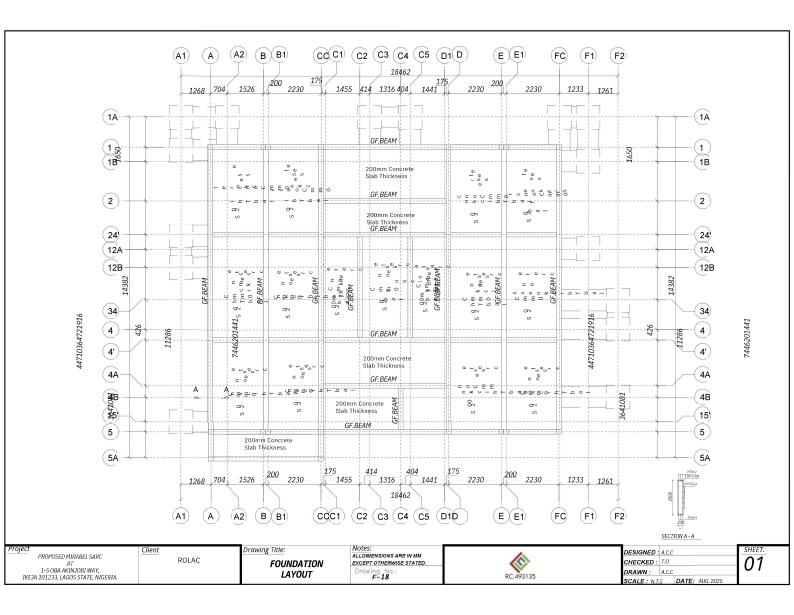


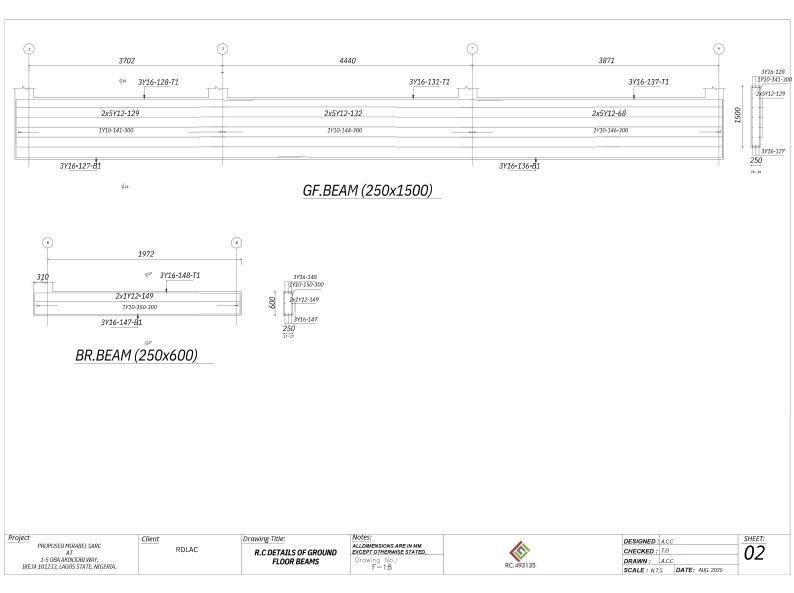


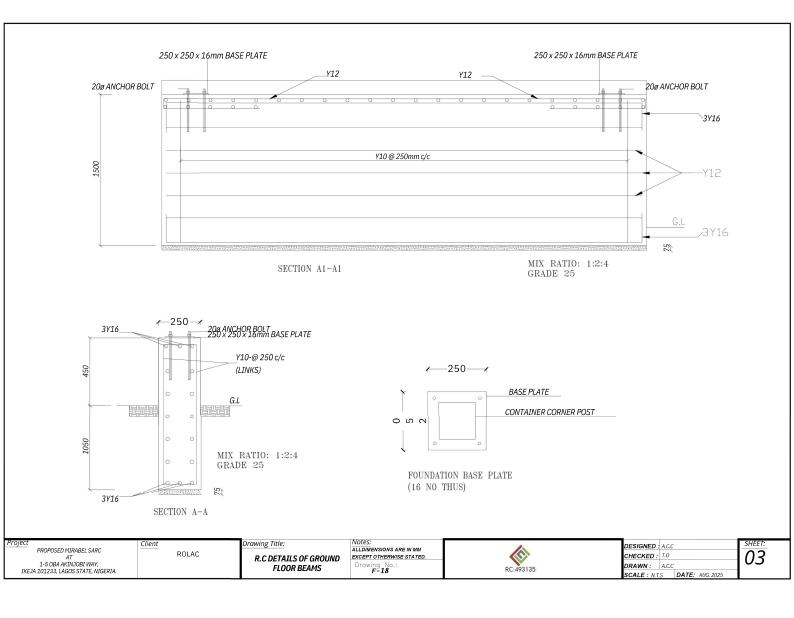


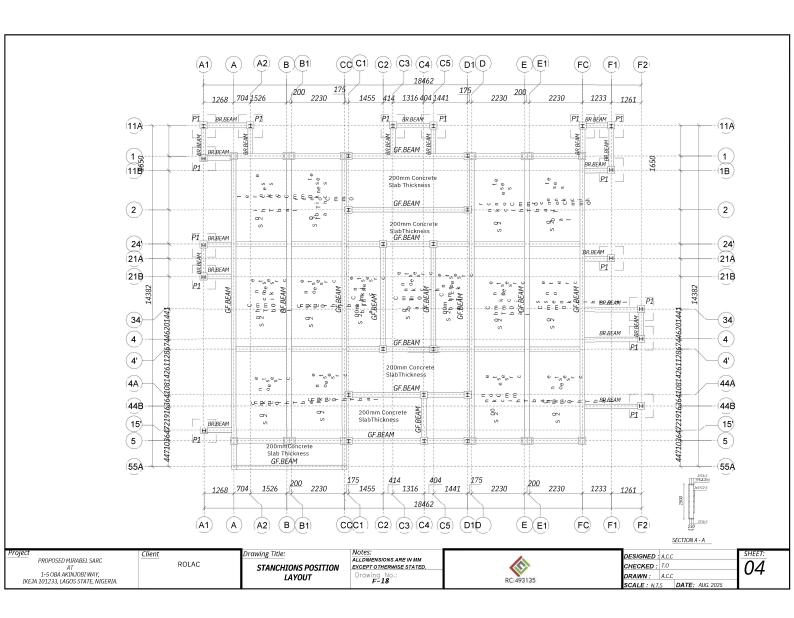
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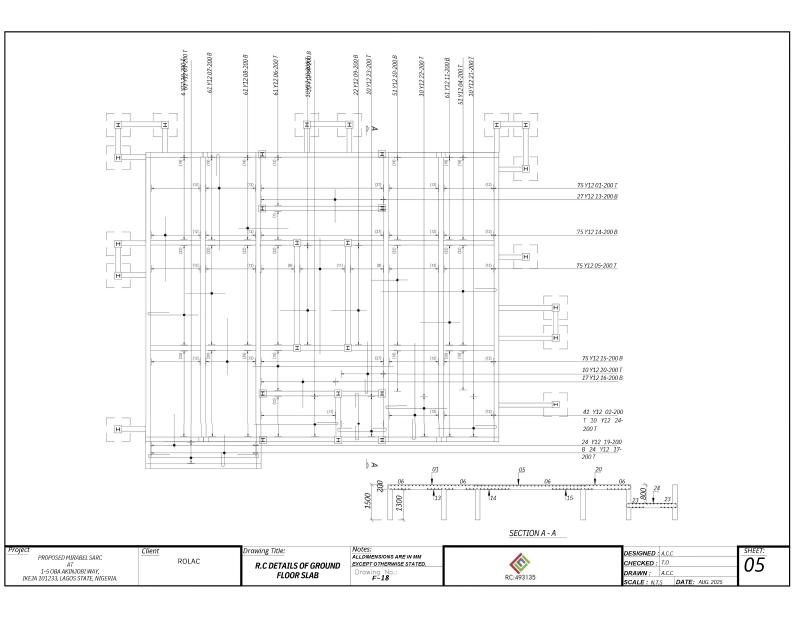


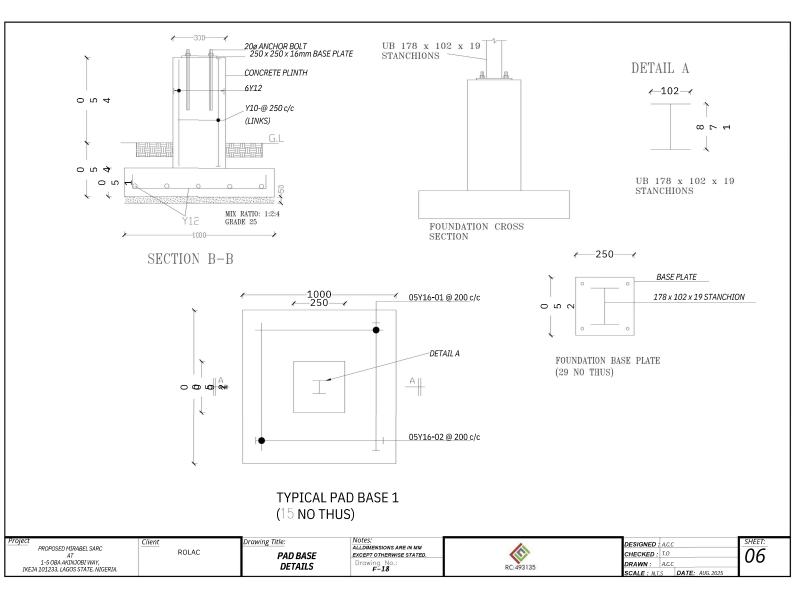


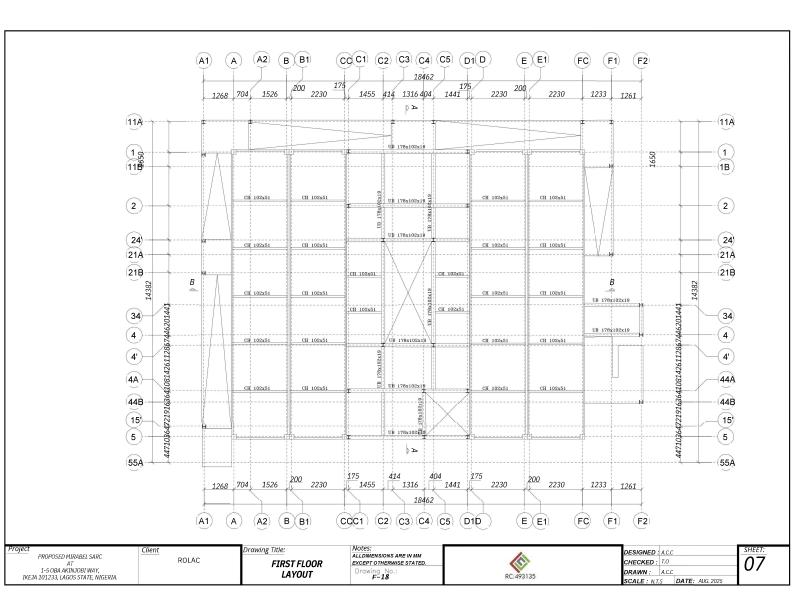


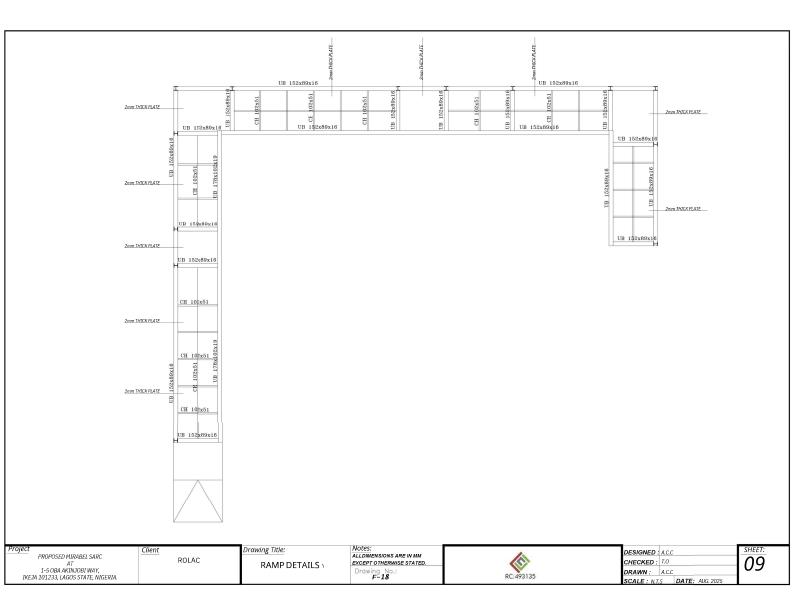


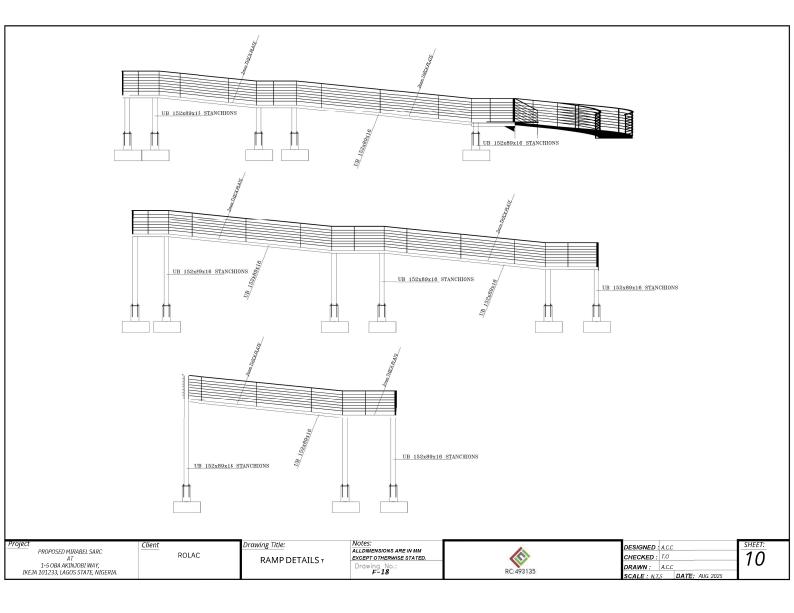


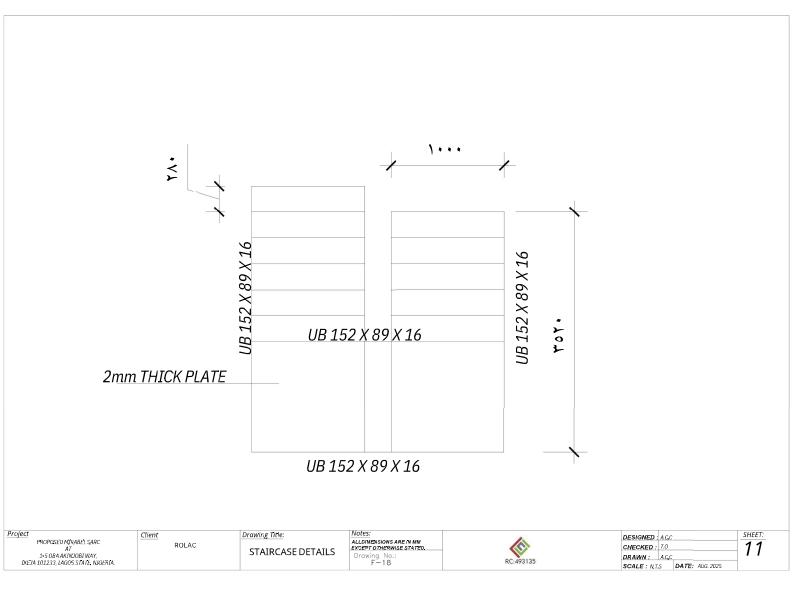


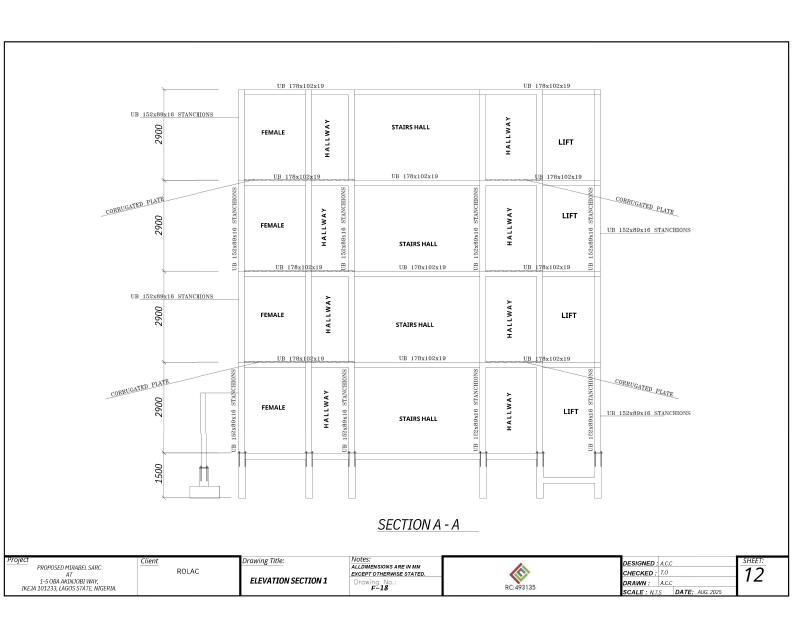


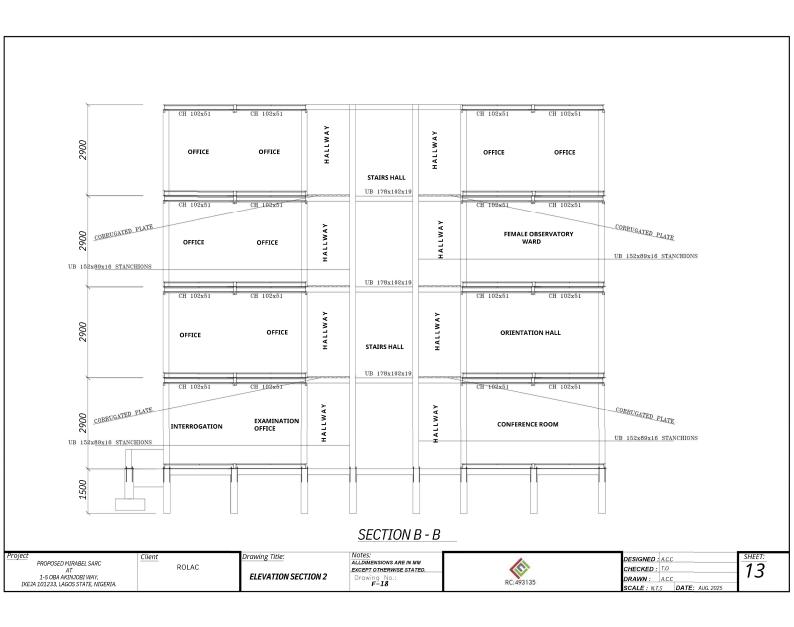










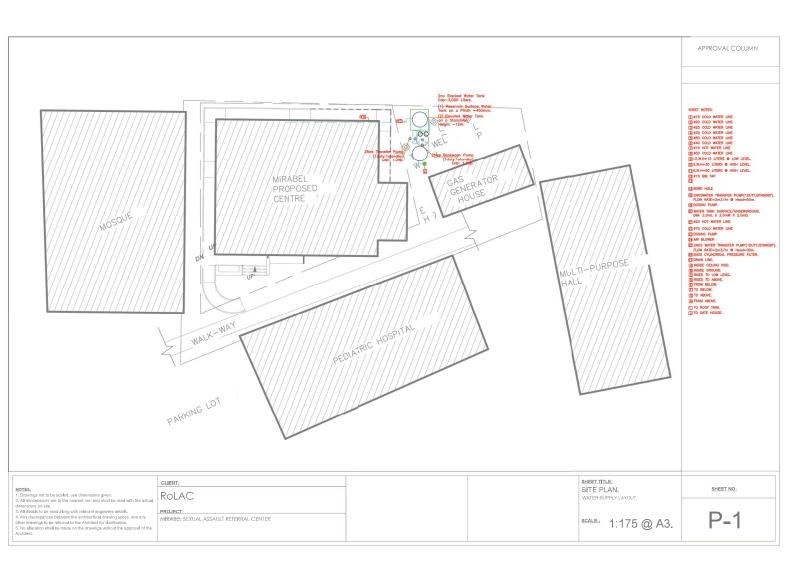


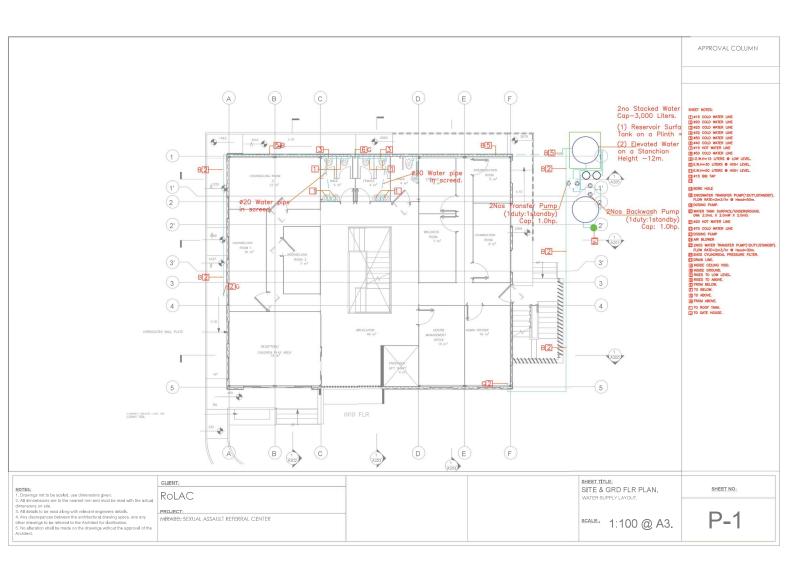
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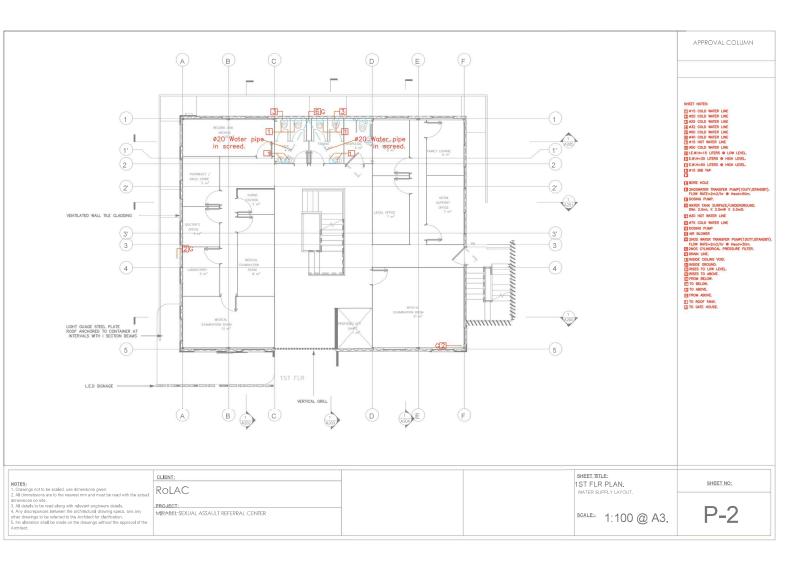


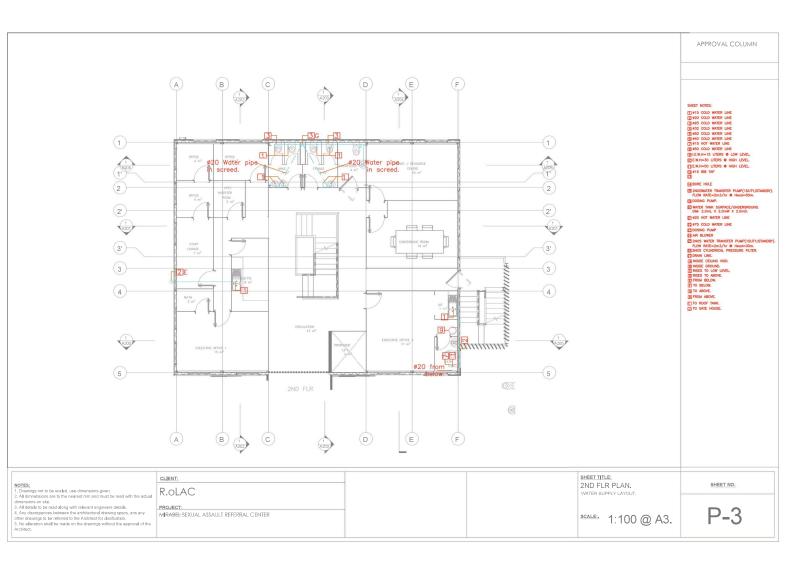


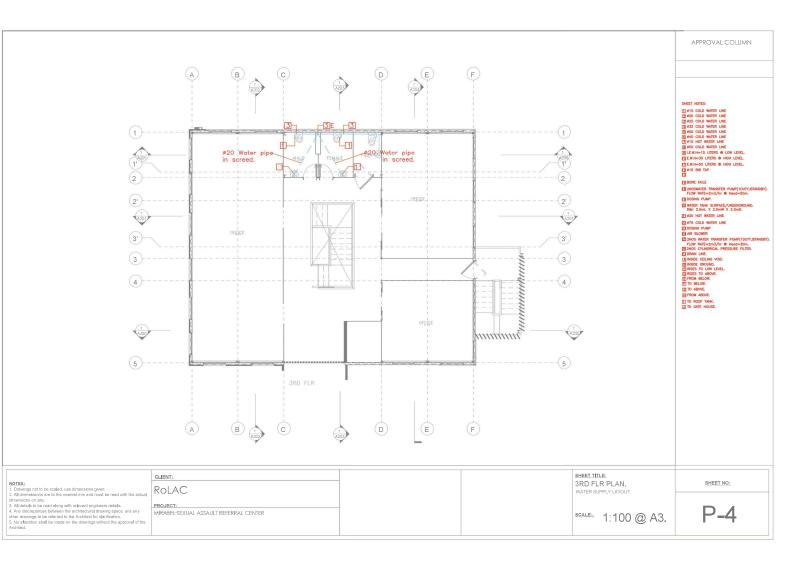
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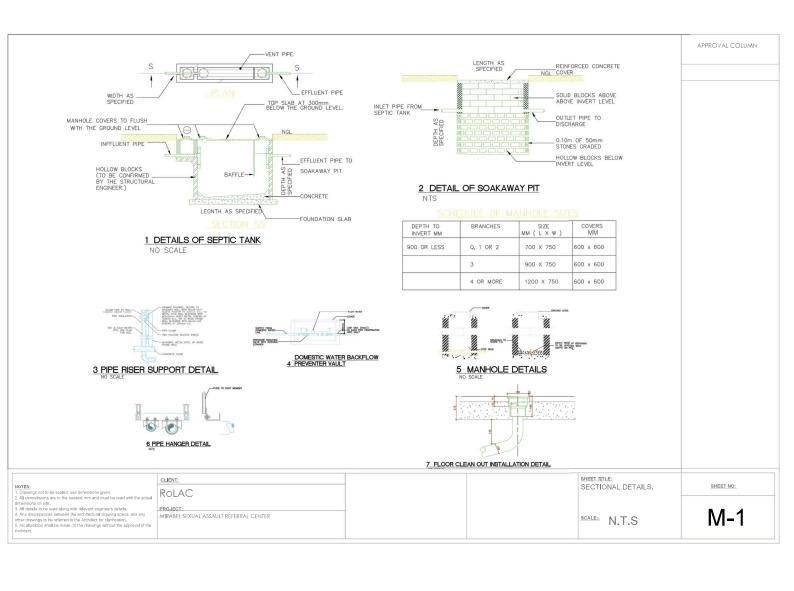


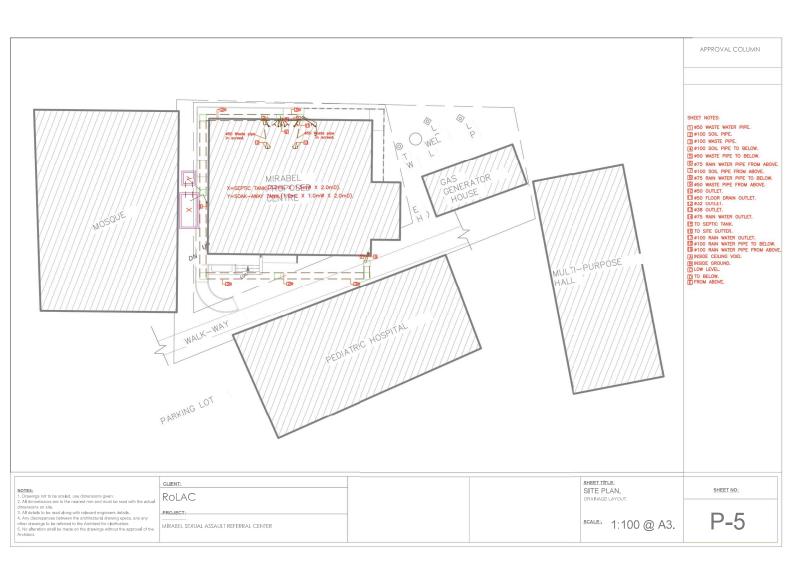


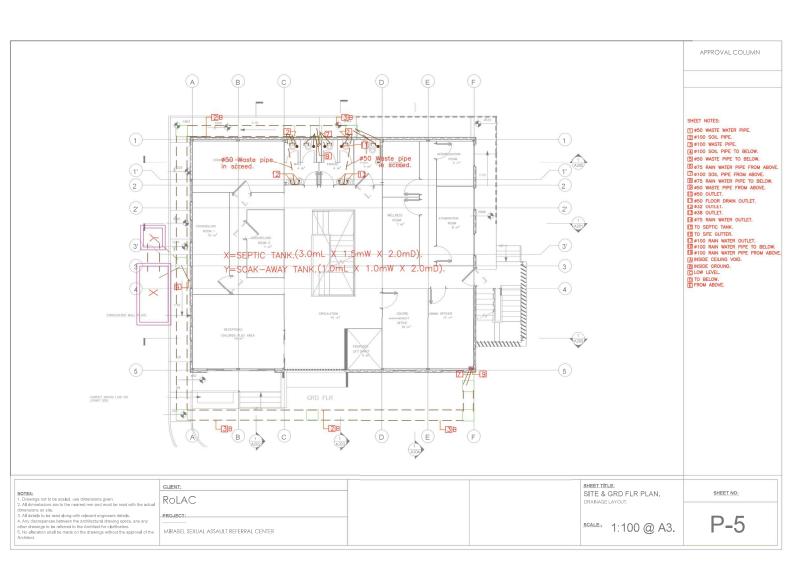


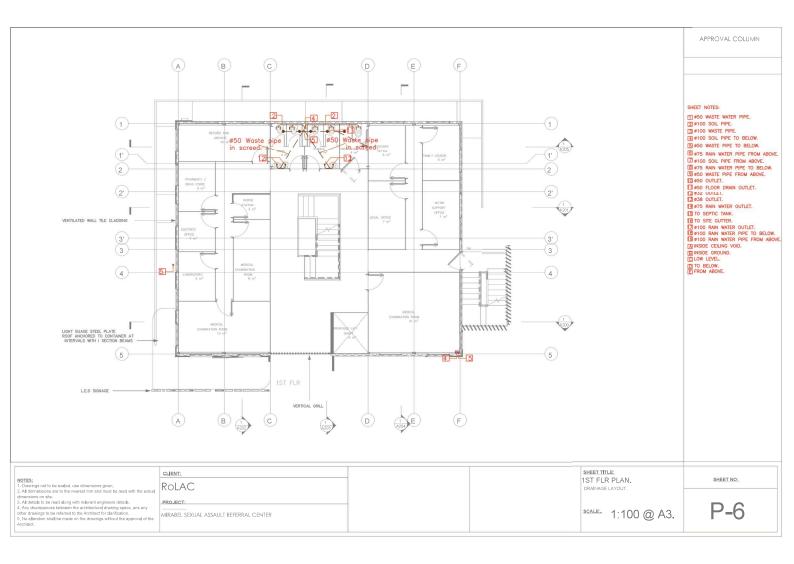


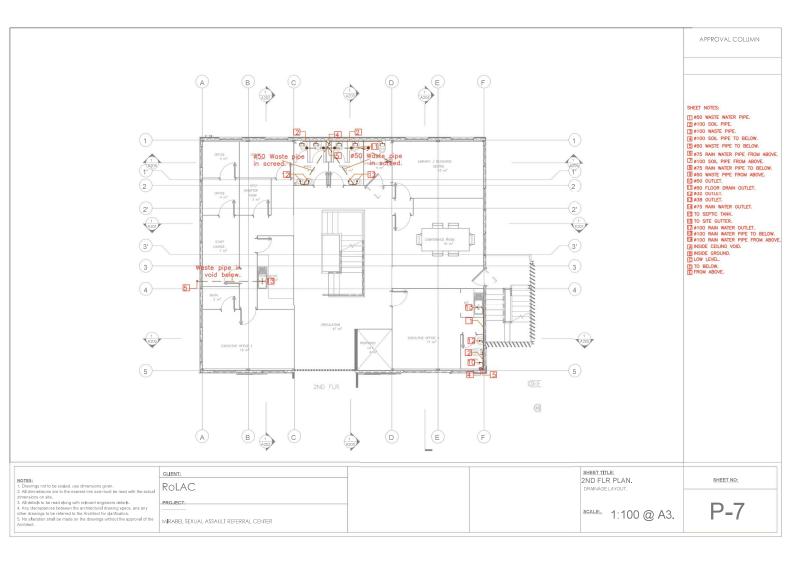


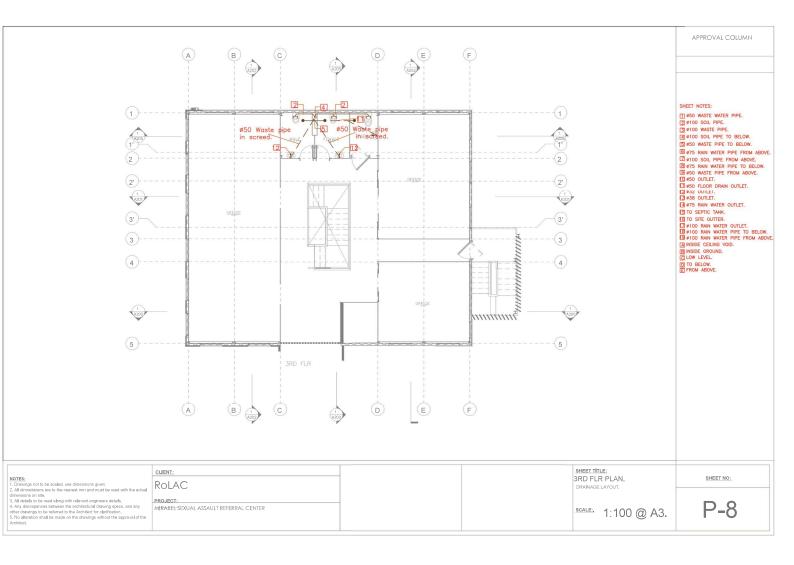


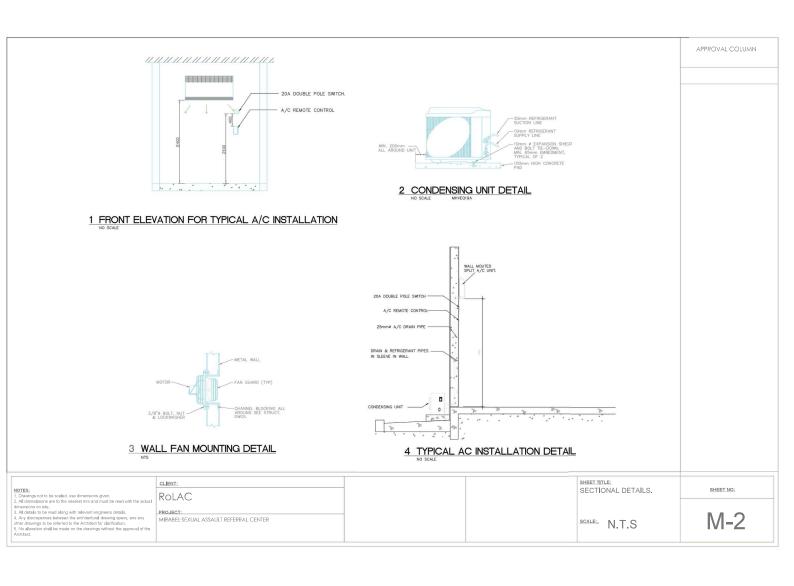


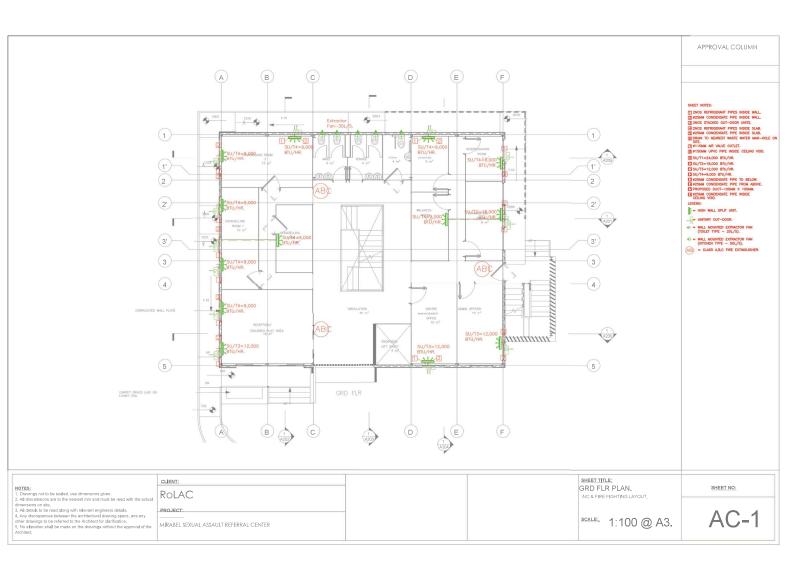


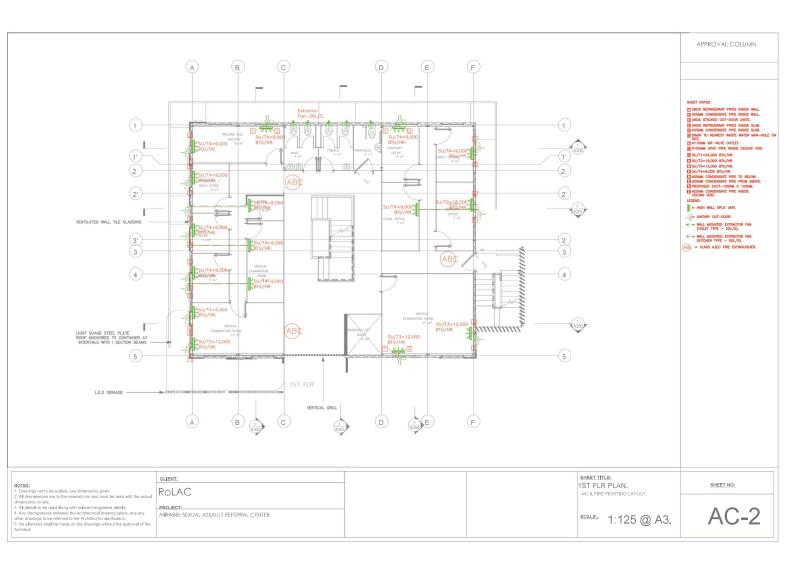


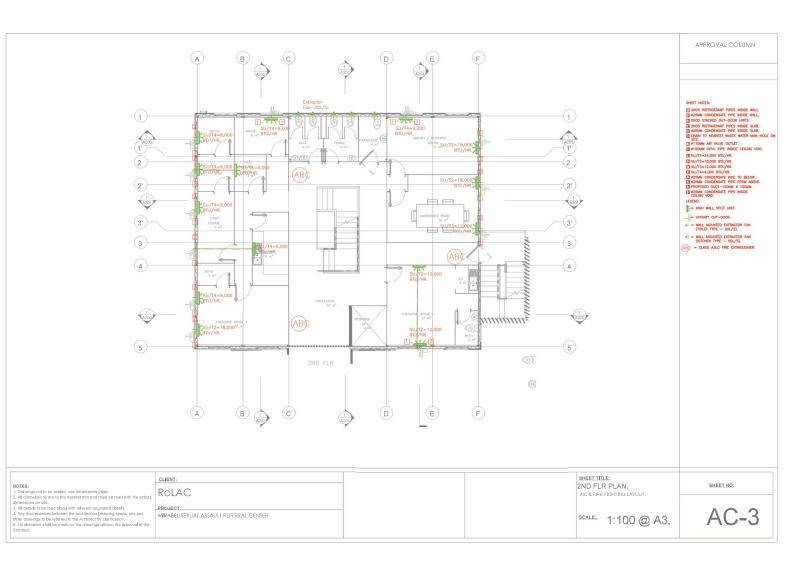


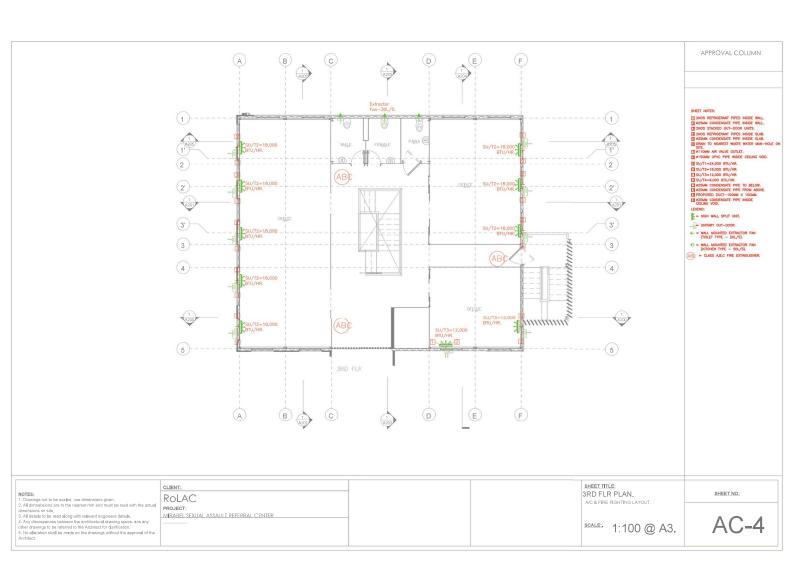












ANNEX 5A-SOIL REPORT

REPORT

ON

SUB-SOIL (GEOTECHNICAL) INVESTIGATION

AT THE SITE OF DESIGN AND CONSTRUCTION

OF

MIRABEL CENTER (LASUTH)

A PROPOSED STRUCTURAL DEVELOPMENT

 \mathbf{AT}

LASUTH, IKEJA, LAGOS STATE

SEPTEMBER, 2025

BY
DEXTOL GLOBAL GEOPHYSICAL
IBADAN, OYO STATE

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Conclusion and Recommendation
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Appendix

1.0 INTRODUCTION

Our Client **MIRABEL CENTER LASUTH** desires to carry out a proposed structural development on the property located at LASUTH, Ikeja, Lagos state.

In other to carry out a safe and economic design and construction of the proposed building structure at the site, it is necessary to determine the nature, strength and suitability of the sub-soil at the project site to carry the proposed development.

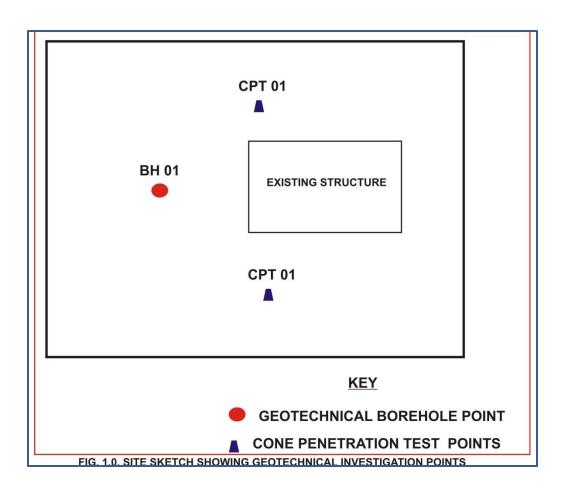
Consequently, our firm, *Dextol Global Geophysical was* commissioned by the client to carry out the sub-soil (geotechnical) investigation at the project site at LASUTH, Ikeja, Lagos state.

This is the report of the geotechnical investigation that was carried out at the project site. It is prepared and presented on completion of the field investigation. It contains the description of the field tests and the methodology for data acquisition and deductions made from observation of site physical features e.g. topography, groundwater condition.

2.0 LOCATION OF THE PROJECT SITE

The project site is located on a piece of land at LASUTH, Ikeja, Lagos state.

Accessibility to the project site is fair via a paved road. The project site generally low-lying. The sketch map of the project site showing the investigation points is presented in Figure 1.



3.0 PURPOSE AND SCOPE OF WORK

The main objective of the site investigation was to carry out sub-soil test to determine the sub-soil stratigraphy of the project site and provide engineering parameters that will guide the Structural Engineer in the design of the foundation of the proposed building construction at the site.

Consequently, the works embodied in this contract include the following:

- Carrying out ONE (1) Standard Penetration Test to refusal point at depth
- Carrying out TWO (2) Cone Penetration Test to refusal point at depth
- Analyses, preparation and submission of sub-soil (geotechnical) investigation report. The schedule of investigation at the project site is presented below:

TABLE 1: SCHEDULE OF INVESTIGATION

Investigation Type	Depth	Groundwater Level						
	(m)	(m)						
STANDARD PENETRATION TEST								
Geotechnical Borehole 01	-9.00m	SWL						
CONE PENETRATION TEST								
Cone Penetrometer Test 01	-2.00m	SWL						
Cone Penetrometer Test 02	-2.50m	SWL						

4.0 GENERAL GEOLOGY OF THE PROJECT SITE

The geology of Lagos State is mainly sedimentary of tertiary and quaternary sediments. Tertiary sediments are unconsolidated sandstones, grits with mudstone band and sand with layers of clay. Quaternary sediments are recent deltaic sands, mangrove swamps and alluvium near the coast. The state is located on sedimentary rock mainly of sand and alluvium. The major soil groups are juvenile, organic- hydromorphic and ferrallitic soils. The geologic succession in Lagos spans through the Cretaceous Abeokuta Formation, which unconformably overlies the rocks of the Basement Complex, to the Quaternary Deltaic Plain Sands. The Benin Formation consists largely of sands/ sandstones with lenses of shales and clays.

5.0 FIELDWORK

The fieldwork was carried out on the 5th of SEPTEMBER, 2025 and it involved carrying out ONE (1) Geotechnical borehole Points and TWO (2) Cone Penetration Test Points.

Geotechnical Borehole

Geotechnical borehole were drilled for soil sampling (disturbed and undisturbed) to a refusal depth of -9.00 m below the natural ground level at the site of investigation. Groundwater was not encountered in the borehole drilled at the site. The strata-log of the geotechnical boreholes is presented in Figures 2 while the summary of the descriptive logs of the geotechnical borehole are presented in section 6.0.

Geotechnical borehole is a very valuable method of sub-soil investigation for the assessment of soil strength and deformability characteristics. It involves collecting disturbed and undisturbed soil samples from the pits at specific intervals for laboratory tests such as grain size analysis, consistency limits, quick undrained triaxial test, and oedometer consolidation test hence providing parameters that will aid in determining the strength and deformation characteristics of the sub-soil.

The preparation for and methods of taking samples together with their size, preservation and handling were in accordance with the British Standard Code of Practice for Site Investigations B.S.5930:1981.

All the soil samples obtained were examined and registered in terms of color, consistency, texture and structure.

The soil samples were taken to the engineering soil laboratory and subjected to soil classification test and strength tests which includes: triaxial test, consolidation test, sieve analysis and Atterberg's limit.

6.0 FIELD TESTS RESULT AND ANALYSIS

6.1 Sub-Soil Stratification

The sub-soil stratification of the geotechnical borehole point is described below:

BH 01

0.00 - 0.25m	- Dark brown gravely Silty SAND
0.25 - 0.75m	- Dark brown CLAYEY SAND
0.75 - 2.25m	- Brownish firm SANDY CLAY with occasional gravel.
2.25 - 3.00m	- Brownish stiff SANDY CLAY with occasional gravel.
3.00 - 4.50m	- Reddish brown ferrugenized Lateritic CLAY with Hardpan
	inclusions
4.50 - 6.00m	- Reddish brown ferrugenized very stiff Lateritic CLAY with
	Hardpan Inclusions
6.00 - 9.00m	- Hardpan/decomposed rock

Groundwater was not encountered in the Borehole drilled at this location.

6.2. Soil Penetration Resistance based on SPT 'N' Values.

The Standard Penetration Test (SPT) was carried out in the geotechnical borehole in accordance with procedures set out in BS. 1377:1975, Test 19 in subsoil while drilling progressed.

As seen in Table 2 presented below, the results of the SPT data recorded for the sub-soil at this location for the proposed development was quite similar, varying between medium and high values in the sub-soil at this site with depth.

TABLE 2: THE SUMMARY OF SPT 'N' VALUES FROM THE GEOTECHNICAL BOREHOLES AT THE PROJECT SITE

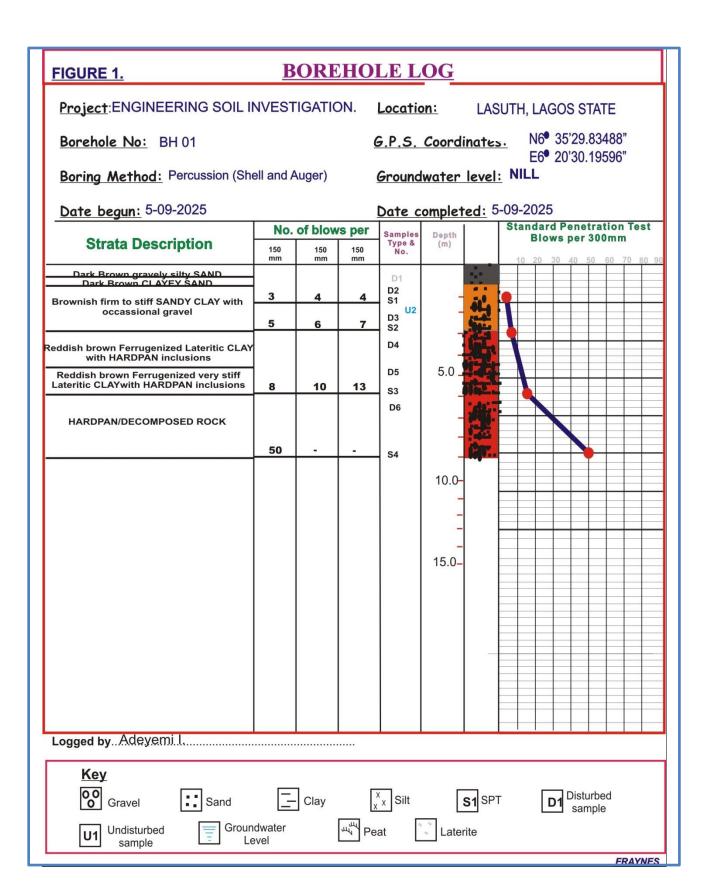
Depth (m)	BH 01
(m)	(GWL: -NILL)
• 1.50	8
-3.00	13
-6.00	23
-9.00	>50

Consequently, the sub-soil at the shallow depth is generally composed of hard soil materials on the weathered bedrock. These translate to appreciable bearing capacity values in the sub-soil at this site.

These values were thereafter corrected for overburden pressure, hammer efficiency, drill rod type, sampler type and borehole diameter to get the generalized 'N_{corr}' value below:

TABLE 3: THE SUMMARY OF SPT 'N_{corr}' VALUES FROM THE GEOTECHNICAL BOREHOLES AT THE PROJECT SITE

Depth	BH 01	BH 01
(m)	SPT 'N' Value	SPT 'N _{corr} ' Value
- 1.50	8	6
-3.00	13	10
-6.00	23	18
-9.00	>50	>40



DEUTSCH CONE PENETROMETER TEST GRAPHS (CPT 01 TO CPT 02)

FIGURE 1

DEUSTCH CONE PENETROMETER TEST

PROJECT: ENGINEERING SOIL INVESTIGATION

CLIENT: MIRABEL CENTER LASUTH

LOCATION: LASUTH, LAGOS STATE

REMARKS: STOPPED AT -2.00 m MAXIMUM ANCHORAGE

DATE: SEPTEMBER, 2025

TEST NO: 01

MACHINE: 2.5 Ton

GROUNDWATER LEVEL: NILL

Depth (m)	Cone	Friction
Deptii (iii)	Resistance	(Kg/cm²)
0	0	(Ng/CIII)
-0.25	5	
-0.5	8	
-0.75	10	
-1	20	
-1.25	50	
-1.5	85	
-1.75	90	
-2	100	
-2.25	100	
-2.5		
-2.75		
-3		
-3.25		
-3.5		
-3.75		
-4		
-4.25		
-4.5		
-4.75		
-5		
-5.25		
-5.5		
-5.75		
-6		
-6.25		
-6.5		
-6.75		
-0.73	-	
-7.25		
-7.5		
-7.75		
-8		
-8.25		
-8.5		
-8.75		
-9		
-9.25		
-9.5		
-9.75		
-10		
-10.25		
-10.5		
-10.75		
-11		
-11.25		
-11.5		
-11.75		

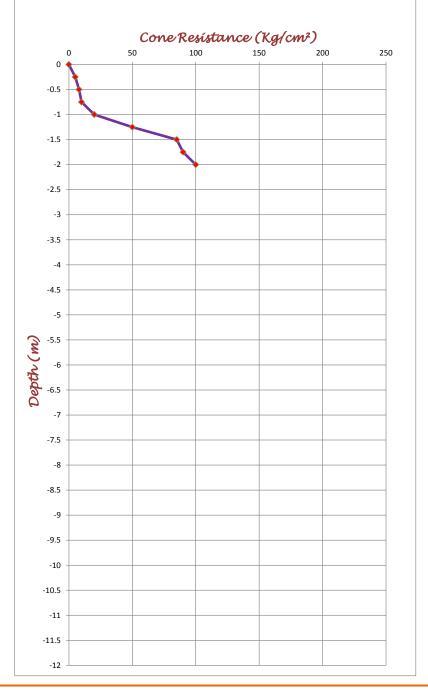


FIGURE 2

DEUSTCH CONE PENETROMETER TEST

PROJECT: ENGINEERING SOIL INVESTIGATION

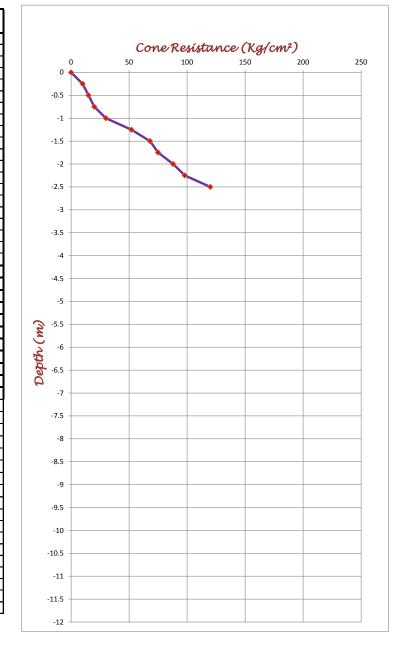
CLIENT: MIRABEL CENTER LASUTH LOCATION: LASUTH, LAGOS STATE

REMARKS: STOPPED AT -2.50 m MAXIMUM ANCHORAGE

DATE: SEPTEMBER, 2025

TEST NO: 02
MACHINE: 2.5 Ton
GROUNDWATER LEVEL: NILL

Depth (m)	Cone	Friction							
	Resistance	(Kg/cm²)							
0	0								
-0.25	10								
-0.5	15								
-0.75	20								
-1	30								
-1.25	52								
-1.5	68								
-1.75	75								
-2	88								
-2.25	98								
-2.5	120								
-2.75 -3									
-3.25	1								
-3.5 -3.75									
-5.75									
-4.25									
-4.5									
-4.75									
-5	 								
-5.25									
-5.5	-								
	-								
-5.75 -6	-								
-6.25									
-6.5									
-6.75									
-0.75	-								
-7 -7.25	-								
-7.25 -7.5	-								
-7.75									
-8									
-8.25									
-8.5									
-8.75									
-9									
-9.25									
-9.5									
-9.75									
-10									
-10.25									
-10.5									
-10.75									
-11									
-11.25									
-11.5									
-11.75									
-12									



6.2. CONE PENETRATION TEST INTERPRETATION

CPT 01: The soil resistance value (kg/cm²) at -0.00 m to 1.00 m is 0 kg/cm² 20 kg/cm². The values increased from 50 kg/cm² to 90 kg/cm² at -1.00 m to -1.75m (STIFF TO VERY STIFF CLAY) and 100 kg/cm² at a refusal depth of -2.00 m (VERY STIFF CLAY), hence translating to an appreciable bearing capacity of the sub-soil strata.

Implication: The soil resistance range within depth -0.00 to -1.00m shows that the soil layer is relatively firm/moderate. The soil shows significantly better resistance values at -1.25m to a competent load bearing-stratum at the refusal point of -2.00m which infers adequate strength, suggesting this zone can support moderate to heavy loads.

CPT 02: The soil resistance value (kg/cm²) at -0.00 m to 1.00 m is 0 kg/cm² 30 kg/cm². The values increased from 52 kg/cm² to 98 kg/cm² at -1.00 m to -2.25m (STIFF TO VERY STIFF CLAY) and 120 kg/cm² at a refusal depth of -2.50 m (VERY STIFF CLAY), hence translating to an appreciable bearing capacity of the sub-soil strata.

Implication: The soil resistance range within depth -0.00 to -1.00m shows that the soil layer is relatively firm/moderate. The soil shows significantly better resistance values at -1.25m to a competent load bearing-stratum at the refusal point of -2.50m which infers adequate strength, suggesting this zone can support moderate to heavy loads.

6.3 Laboratory Analyses

Laboratory analyses such as soil classification tests – Grain size analysis and Atterberg, quick undrained triaxial test, oedometer consolidation tests were carried out on the representative soil samples from the ONE (1) geotechnical boreholes drilled at the project site.

KEY SOIL PARAMETERS

- Bulk density 2.08 Mg/m³
- Cohesion 65 KN/m²
- Angle of internal friction- 9⁰
- % fines 8 % 25 %
- % Sand- 66% -89%
- % Gravel 1% 13%
- Soil classification: SC
- Plasticity index- 21% 22%
- Liquid limit: -35% 38%
- Compressibility: medium

					S	UMM	ARY C	OF LA	BORA	TORY	TEST	RESU	LTS OF	LASUT	H, LAGO	OS STAT	ΓΕ			
BH/	T	Depth	Particle	e Size D	istributi	on	Nat.	A	Atterberg		γВ	SG		Quick Und	rained					
Sample	Y			% Pas	ssing:		M.C		Limits					Triaxial Te	ests.		Soil Class (UCS)	Oedometer Cor	solidatio	ı Tests.
No.	P E	(m)	2.00 Mm	425 μm	63 μm	2 μm	%	LL % Dw	PL % dw	PI % D	Mg/m^3		Dia. Type (mm)	σ_3 KN/m ²	Cu KN/m²	Ø Deg.	(003)	Stress Range KN/m ²	M _v m ² /M N	C _v m²/yr
										W										
BH 01																				
	D	0.75	99	91	10		12	35	14	21										
	U	1.00	98	90	25		13	38	17	21	2.08		38	100	65	9		50 - 100	0.154	2.8
	D	2.25	91	89	20		13							200				100 - 200	0.159	2.9
	D	3.00	90	86	24									400				200 - 400	0.143	3.0
	ъ		89	88	23		10	37	15	22								400 - 800	0.145	3.0
	D	5.75	87	85	8		12	3/	15	22										
	D	7.50	07	65	0		14													
	-																			
																				•

7.0 DEDUCTIONS AND RECOMMENDATIONS

Generally, the sub-soil stratification at this site is as shown by the geotechnical borehole drilled on the project site.

The lithology consists essentially of Dark brown gravely silty SAND, Dark Brown CLAYEY SAND, Brownish firm SANDY CLAY, Brownish stiff SANDY CLAY with occasional gravel, and Reddish Brown Ferrugenized Lateritic SANDY CLAY with Hardpan inclusions. Groundwater was not encountered in the drilled geotechnical borehole.

Soil characteristics

The sub-soil at this location is classified as **INORGANIC CLAYS** (**CH**) of high plasticity and **SC** based on the Unified Soil Classification System (**UCS**). The Atterberg's Limits values are moderate with plasticity Index between 21% to 22%, and liquid limits values ranged between 35 % to 38 %. From the Atterberg limits results it can be deduced that the soil is of medium compressibility. Gravel content is between 1 % to 13 %, while the sand content varies between 66 % and 89 %. The fines (silt and Clay) content varied between 8% and 25%.

Groundwater

Groundwater was encountered at surface level in the geotechnical boreholes drilled during this investigation which could be sampled or tested. However, as a result of the appreciable number of fines in the subsoil, hence there can be a softening of the soil with increase in moisture content.

Design/Allowable bearing Pressure

In other to arrive at the bearing pressure values for the design of the foundation of the proposed structural development at the project site, the allowable bearing pressure and settlement estimates were carried out and used to compute for various footing sizes (1.00 m, 1.50 m, 2.00 m, 3.00 m), using the data generated from the field (In-situ) tests and the laboratory oedometer consolidation settlement tests.

The following **design allowable bearing pressure** (**ABP**) values and settlement were computed for the sub-soil at this site. The result of the Computations is presented in table 8 below.

The settlement of the soil is within the range of 4.77 mm to 90.63 mm.

TABLE 8: ALLOWABLE BEARING PRESSURE (ABP) AND SETTLEMENT ESTIMATES FOR SHALLOW FOUNDATION AT THE PROJECT SITE

M _V	σ	B (m) = 1.5		B (m) = 2.25		B (m) = 2		B (m) = 3	
(m2/MN)	(KN/m2)	H (m)	S (mm)	H (m)	S (mm)	H (m)	S (mm)	H (m)	S (mm)
	30	1	4.77	1.5	7.155	2	9.54	3	14.31
	40	1	6.36	1.5	9.54	2	12.72	3	19.08
	50	1	7.95	1.5	11.925	2	15.9	3	23.85
	60	1	9.54	1.5	14.31	2	19.08	3	28.62
	70	1	11.13	1.5	16.695	2	22.26	3	33.39
	80	1	12.72	1.5	19.08	2	25.44	3	38.16
	90	1	14.31	1.5	21.465	2	28.62	3	42.93
0.159	100	1	15.9	1.5	23.85	2	31.8	3	47.7
	110	1	17.49	1.5	26.235	2	34.98	3	52.47
	120	1	19.08	1.5	28.62	2	38.16	3	57.24
	130	1	20.67	1.5	31.005	2	41.34	3	62.01
	140	1	22.26	1.5	33.39	2	44.52	3	66.78
	150	1	23.85	1.5	35.775	2	47.7	3	71.55
	170	1	27.03	1.5	40.545	2	54.06	3	81.09
	180	1	28.62	1.5	42.93	2	57.24	3	85.86
	190	1	30.21	1.5	45.315	2	60.42	3	90.63

PAD FOUNDATION DESIGN

DEPTH (meters)	FOOTING DIMENSIONS (L X B) (meters)	ABP (KN/m²)	SETTLEMENT (mm)
1.50	1.0 X 1.0 2.0 X 1.5	120	28.62 31.01
	1.5 X 1.5 3.0 X 2.0		34.98 38.16

The settlement of the sub-soil ranged from 5.13mm to 97.47mm, hence suitable for SHALLOW FOUNDATION as it is within the maximum tolerable settlement limits (≤40mm in SANDY soils and ≤65mm on CLAYEY soils).

Consequently, an allowable bearing pressure (ABP) value of 120 KN/m² may be adopted for the design of the PAD foundation of the proposed structural development at this site at depth of -1.50 meters, provided it satisfies the Structural Engineers requirements for the proposed building.

It is advisable that the pads be tied together to mitigate settlement and ensure better rigidity of the structure.

Foundation Construction Works

Foundation excavation trench opened at this site during construction must be completely backfilled and compacted quickly to protect it from moisture and water on completion of foundation works. Topographically, the project site is on a low-lying terrain.

For concrete works, we recommend the use of rich, dense mix of ordinary Portland cement to satisfy the standard equivalent to CLASS 2 in the Code of Practice for foundations B.S. 8004: 1984, together with good embedment of reinforcement in all concrete structures at this project site.

The observations and deductions made in this report are based on the ground condition as revealed by the position of the geotechnical borehole and soil sampling which were carried out at the project site as well as visual site observation and laboratory tests on the recovered disturbed and undisturbed soil samples.

The structural/foundation design Engineer for the project shall appraise the prevailing sub-soil conditions as presented herein along with the proposed structures foundation option to arrive at the best choice of foundation type for the proposed development at the site.

PREPARED BY:

ADEYEMI IFEOLUWA O (MNAEGE, MNMGS)



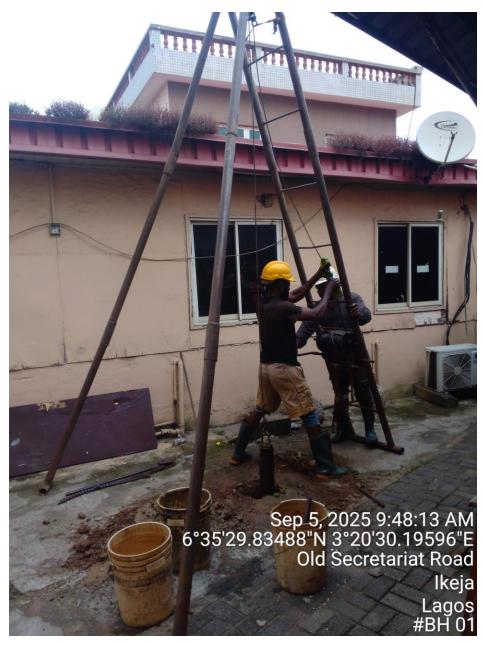
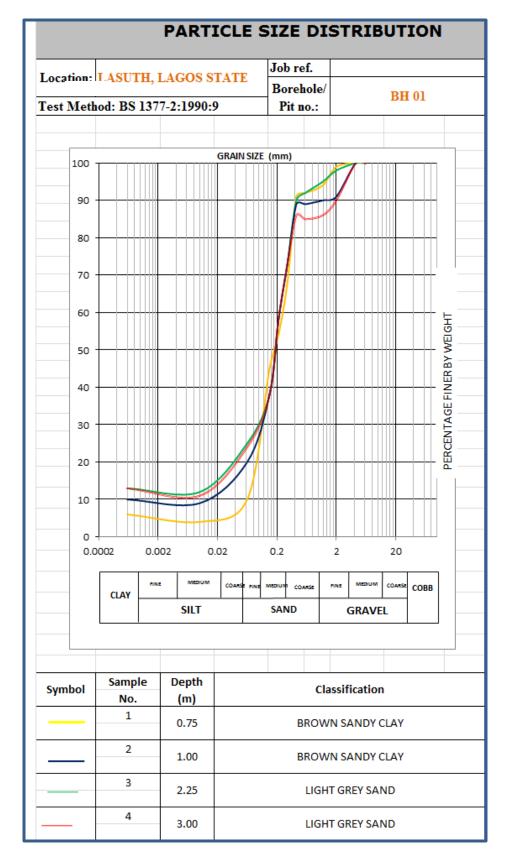


Fig 1.0: GEOTECHNICAL INVESTIGATION BEING CARRIED OUT ON SITE

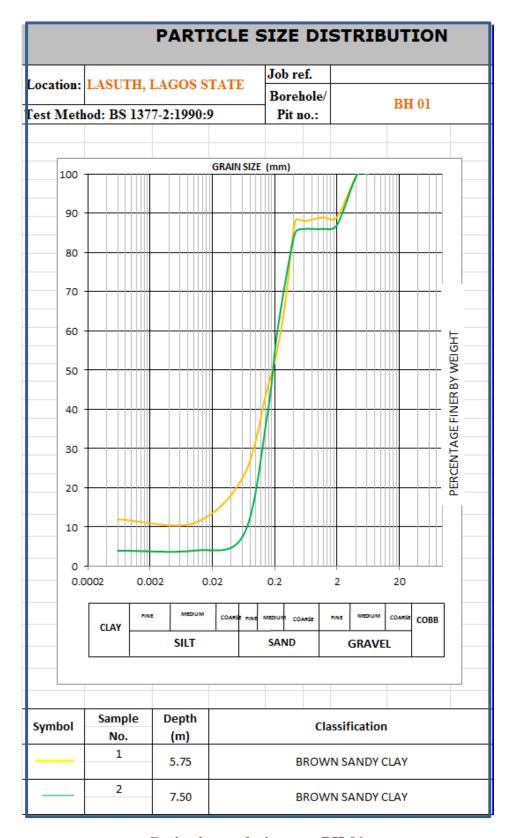


Fig 2.0: GEOTECHNICAL INVESTIGATION BEING CARRIED OUT ON SITE

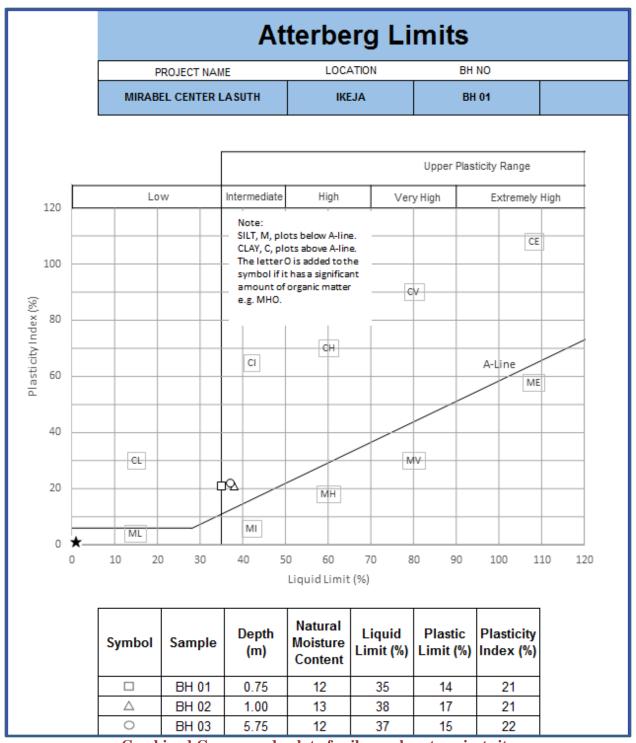
APPENDIX



Grain size analysis curve BH 01



Grain size analysis curve BH 01



Combined Cassagrande plot of soil samples at project site

LABORATORY QUICK UNDRAINED COMPRESSION TEST

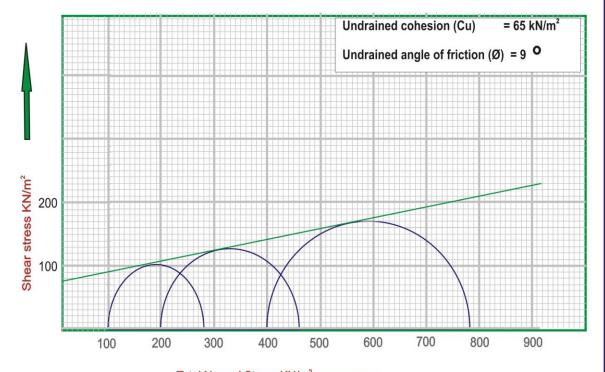
MOHR'S CIRCLE DIAGRAM

PROJECT: MIRABEL CENTER, LASUTH

BH NO..: BH 01

SAMPLE NO: 3 SAMPLE TYPE: Undisturbed Sample

DEPTH: -1.00 m DATE: SEPTEMEBR. 2025



Total Normal Stress KN/m² Max Deviation Stress G-1

Cell Pressure σ_3 (KN/m²)	Max. Deviation Stress σ_1 - σ_3 (KN/m^2)	Total Normal Stress (KN/m 2) σ_1
100	170	280
200	260	460
400	380	780

THE END

THANK YOU

ANNEX 5BSUBSOIL GEOTECHNICAL INVESTIGATION

REPORT

ON

SUB-SOIL (GEOTECHNICAL) INVESTIGATION
AT THE SITE OF DESIGNAND CONSTRUCTION

OF

MIRABEL CENTER (LASUTH)

A PROPOSED STRUCTURAL DEVELOPMENT

AT

LASUTH, IKEJA, LAGOS STATE

SEPTEMBER, 2025

BY
DEXTOL GLOBALGEOPHYSICAL
IBADAN, OYO STATE

TABLE OF CONTENT

Introduction	 2
Location of Project Site	2
Site Sketch	4
Purpose and Scope of work	5
General Geology of Project Site	7
Fieldwork	7
Fieldwork and Interpretations	7-16
Laboratory analysis	17-28
Design/Allowable bearing pressure	29
Conclusion and Recommendation	
PLATES	
Appendix	

1.0 INTRODUCTION

Our Client *MIRABEL CENTER LASUTH* desires to carry out a proposed structural development on the property located at LASUTH, Ikeja, Lagos state.

In other to carry out a safe and economic design and construction of the proposed building structure at the site, it is necessary to determine the nature, strength and suitability of the sub-soil at the project site to carry the proposed development.

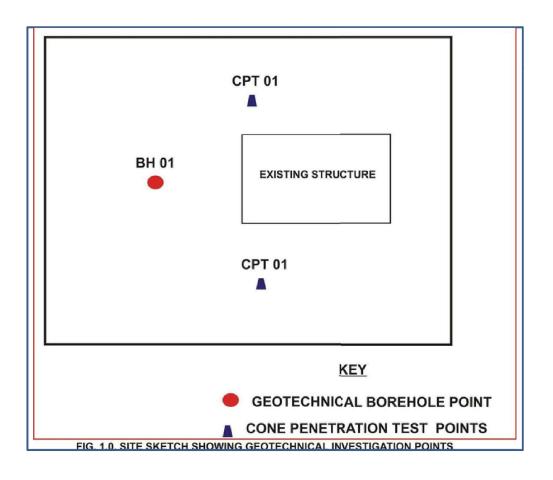
Consequently, our firm, *Dextol Global Geophysical was* commissioned by the client to carry out the sub-soil (geotechnical) investigation at the project site at LASUTH, Ikeja, Lagos state.

This is the report of the geotechnical investigation that was carried out at the project site. It is prepared and presented on completion of the field investigation. It contains the description of the field tests and the methodology for data acquisition and deductions made from observation of site physical features e.g. topography, groundwater condition.

2.0 LOCATION OF THE PROJECT SITE

The project site is located on a piece of land at LASUTH, Ikeja, Lagos state.

Accessibility to the project site is fair via a paved road. The project site generally low-lying.. The sketch map of the project site showing the investigation points is presented in Figure 1.



3.0 PURPOSE AND SCOPE OF WORK

The main objective of the site investigation was to carry out sub-soil test to determine the sub-soil stratigraphy of the project site and provide engineering parameters that will guide the Structural Engineer in the design of the foundation of the proposed building construction at the site.

Consequently, the works embodied in this contract include the following:

Carrying out ONE (1) Standard Penetration Test to refusal point at depth
Carrying out TWO (2) Cone PenetrationTest to refusal point at depth
Analyses, preparation and submissionofsub-soil (geotechnical) investigation report. The schedule of investigation at the projectsite is presented below:.

TABLE 1: SCHEDULEOF INVESTIGATION

Investigation Type	Depth (m)	Groundwater Level (m)								
STANDARD PENETRATION TEST										
GeotechnicalBorehole 01	SWL									
CONE PENETRATION TEST										
Cone Penetrometer Test 01	-2.00m	SWL								
Cone Penetrometer Test 02	-2.50m	SWL								

4.0 GENERAL GEOLOGY OF THE PROJECT SITE

The geology of Lagos State is mainly sedimentary of tertiary and quaternary sediments. Tertiary sediments are unconsolidated sandstones, grits with mudstone band and sand with layers of clay. Quaternary sediments are recent deltaic sands, mangrove swamps and alluvium near the coast. The state is located on sedimentary rock mainly of sand and alluvium. The major soil groups are juvenile, organic- hydromorphic and ferrallitic soils. The geologic succession in Lagos spans through the Cretaceous Abeokuta Formation, which unconformably overlies the rocks of the Basement Complex, to the Quaternary Deltaic Plain Sands. The Benin Formation consists largely of sands/ sandstones with lenses of shales and clays.

5.0 FIELDWORK

The fieldwork was carried out on the 5th of SEPTEMBER, 2025 and it involved carrying out ONE (1) Geotechnical borehole Points and TWO (2) Cone Penetration Test Points.

Geotechnical Borehole

Geotechnical borehole were drilled for soil sampling (disturbed and undisturbed) to a refusal depth of -9.00 m below the natural ground level at the site of investigation. Groundwater was not encountered in the borehole drilled at the site. The strata-log of the geotechnical boreholes is presented in Figures 2 while the summary of the descriptive logs of the geotechnical borehole are presented in section 6.0.

Geotechnical borehole is a very valuable method of sub-soil investigation for the assessment of soil strength and deformability characteristics. It involves collecting disturbed and undisturbed soil samples from the pits at specific intervals for laboratory tests such as grain size analysis, consistency limits, quick undrained triaxial test, and oedometer consolidation test hence providing parameters that will aid in determining the strength and deformation characteristics of the sub-soil.

The preparation for and methods of taking samples together with their size, preservation and handling were in accordance with the British Standard Code of Practice for Site Investigations B.S.5930:1981.

All the soil samples obtained were examined and registered in terms of color, consistency, texture and structure.

The soil samples were taken to the engineering soil laboratory and subjected to soil classification test and strength tests which includes: triaxial test, consolidation test, sieve analysis and Atterberg's limit.

6.0 FIELD TESTS RESULT AND ANALYSIS

6.1 Sub-Soil Stratification

The sub-soil stratification of the geotechnical borehole point is described below:

BH 01

0.00 - 0.25m	- Dark brown gravely Silty SAND
0.25 - 0.75m	- Dark brown CLAYEY SAND
0.75 - 2.25m	- Brownish firm SANDY CLAY with occasional gravel.
2.25 - 3.00m	- Brownish stiff SANDY CLAY with occasional gravel.
3.00 - 4.50m	- Reddish brown ferrugenized Lateritic CLAY with Hardpan
	inclusions
4.50 - 6.00m	- Reddish brown ferrugenized very stiff Lateritic CLAY with
	Hardpan Inclusions
6.00 - 9.00m	- Hardpan/decomposed rock

Groundwater was not encountered in the Borehole drilled at this location.

6.2. Soil Penetration Resistance based on SPT 'N' Values.

The Standard Penetration Test (SPT) was carried out in the geotechnical borehole in accordance with procedures set out in BS. 1377:1975, Test 19 in subsoil while drilling progressed.

As seen in Table 2 presented below, the results of the SPT data recorded for the sub-soil at this location for the proposed development was quite similar, varying between medium and high values in the sub-soil at this site with depth.

TABLE 2: THE SUMMARY OF SPT 'N' VALUES FROM THE GEOTECHNICAL BOREHOLES AT THE PROJECT SITE

Depth	BH 01
(m)	(GWL: -NILL)
- 1.50	8
-3.00	13
-6.00	23
-9.00	>50

Consequently, the sub-soil at the shallow depth is generally composed of hard soil materials on the weathered bedrock. These translate to appreciable bearing capacity values in the sub-soil at this site.

These values were thereafter corrected for overburden pressure, hammer efficiency, drill rod type, sampler type and borehole diameter to get the generalized 'Ncorr' value below:

TABLE 3: THE SUMMARY OF SPT 'Ncorr' VALUES FROM THE GEOTECHNICAL BOREHOLES AT THE PROJECT SITE

Depth	BH 01	BH 01
(m)	SPT 'N' Value	SPT 'Ncorr' Value
- 1.50	8	6
-3.00	13	10
-6.00	23	18
-9.00	>50	>40

												_
FIGURE 1.	<u>B</u>	ORE	НО	LE L	<u>OG</u>							Ц
<u>Project</u> :ENGINEERING SOIL INVESTIGATION. <u>Location:</u> LASUTH, LAGOS STATE												
Borehole No: BH 01 G.P.S. Coordinates. N6 [®] 35'29.83488" E6 [®] 20'30.19596"												
Boring Method: Percussion (Shell and Auger) Groundwater level: NILL												
<u>Date begun:</u> 5-09-2025				Date c	omplet	<u>ed:</u> 5						┙
Strata Description	150	of blow	150	Samples Type & No.	Depth (m)			dard Blows			n Test m	1
Dark Brown gravely silty SAND Dark Brown CLAYEY SAND	mm	mm	mm	D1			10	20 30	40	50 60	70 80	90
Brownish firm to stiff SANDY CLAY with occassional gravel	3	4	4	D2 S1 D3	-	al's	1				+	
Reddish brown Ferrugenized Lateritic CLAY with HARDPAN inclusions	5	6	7	S2 D4	-	艺	1					
Reddish brown Ferrugenized very stiff Lateritic CLAYwith HARDPAN inclusions	8	10	13	D5 S3	5.0							
HARDPAN/DECOMPOSED ROCK				D6	-							
	50		-	S4	-	Gr.						=
					10.0-							
					-							
					- 15.0-							
											Ħ	
						_						
Logged by Adeyemi I.												
Key Gravel Sand Clay X X X X Silt S1 SPT D1 Disturbed sample												
U1 Undisturbed sample Groundwater Level Peat Laterite												
											RAYNE	EC.

DEUTSCH CONE PENETROMETER TEST GRAPHS (CPT 01 TO CPT 02)

FIGURE 1

DEUSTCH CONE PENETROMETER TEST

ENGINEERING SOIL INVESTIGATION PROJECT:

MIRABEL CENTER LASUTH CLIENT: LASUTH, LAGOS STATE LOCATION:

STOPPED AT -2.00 m MAXIMUM ANCHORAGE REMARKS:

DATE: SEPTEMBER, 2025

TEST NO: 01 **MACHINE**: 2.5 Ton

GROUNDWATER LEVEL: NILL

Depth (m)	Cone	Friction
	Resistance	(Kg/cm²)
0	0	
-0.25	5	
-0.5	8	
-0.75	10	
-1	20	
-1.25	50	
-1.5	85	
-1.75	90	
-2	100	
-2.25		
-2.5		
-2.75		
3		
-3.25		
-3.5		
-3.75		
-4		
-4.25		
-4.5		
-4.75		
-5		
-5.25		
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-7		
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-7.23		
-7.75		
-8		
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-11.75		
-12		

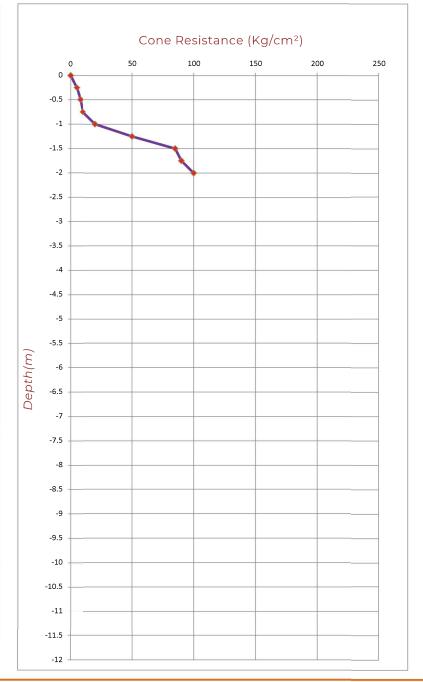


FIGURE 2

DEUSTCH CONE PENETROMETER TEST

ENGINEERING SOIL INVESTIGATION

PROJECT: CLIENT: MIRABEL CENTER LASUTH LOCATION: LASUTH, LAGOS STATE

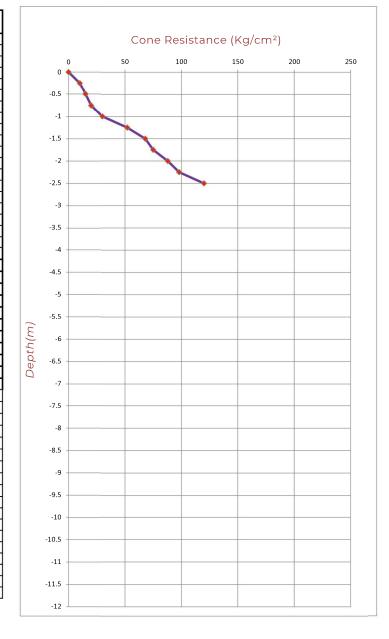
REMARKS: STOPPED AT -2.50 m MAXIMUM ANCHORAGE

DATE: SEPTEMBER, 2025

TEST-NO: 02 **MACHINE**: 2.5 Ton

GROUNDWATER LEVEL: NILL

Depth (m)	Cone	Friction
	Resistance	(Kg/cm²)
0	0	
-0.25	10	
-0.5	15	
-0.75	20	
-1	30	
-1.25	52	
-1.5	- 68	
-1.75	75	
-2		
-2.25	120	
-2.5	120	
-2.75		
3		
-3.25		
-3.5		
-3.75		
-4		
-4.25		
-4.5		
-4.75		
-5		
-5.25		
-5.5		
-5.75		
-6		
-6.25		
-6.5		
-6.75		
-/		
-7.25		
-7.5		
-7.75		
-8		
8.25		
-8.5		
-8.75		
-9		
-9.25		
-9.5		
-9.75		
-10		
-10.25		
-10.25		
-10.75		
-10.75		
-11.25		
-11.5		
-11.75		
-12		



6.2. CONE PENETRATION TEST INTERPRETATION

CPT 01: The soil resistance value (kg/cm) at -20.00 m to 1.00 m is 0 kg/cm² 20 kg/cm. The values increased from 50 kg/cm to 90 kg/cm at -1.200 m to -1.75m (STIFF TO VERY STIFF CLAY) and 100 kg/cmat a refusal depth of -2.00 m (VERY STIFF CLAY), hence translating to an appreciable bearing capacity of the sub-soil strata.

Implication: The soil resistance range within depth -0.00 to -1.00m shows that the soil layer is relatively firm/moderate. The soil shows significantly better resistance values at -1.25m to a competent load bearing-stratum at the refusal point of -2.00m which infers adequate strength, suggesting this zone can support moderate to heavy loads.

CPT 02: The soil resistance value (kg/cm) at 2 0.00 m to 1.00 m is 0 kg/cm 30 kg/cm. 2 The values increased from 52 kg/cm to 2 8 kg/cm at 2 90 m to 2 2.25 m (STIFF TO VERY STIFF CLAY) and 120 kg/cmat a 2 6 fusal depth of 2 2.50 m (VERY STIFF CLAY), hence translating to an appreciable bearing capacity of the sub-soil strata.

Implication: The soil resistance range within depth -0.00 to -1.00m shows that the soil layer is relatively firm/moderate. The soil shows significantly better resistance values at -1.25m to a competent load bearing-stratum at the refusal point of -2.50m which infers adequate strength, suggesting this zone can support moderate to heavy loads.

6.3 Laboratory Analyses

Laboratory analyses such as soil classification tests – Grain size analysis and Atterberg, quick undrained triaxial test, oedometer consolidation tests were carried out on the representative soil samples from the ONE (1) geotechnical boreholes drilled at the project site.

KEY SOIL PARAMETERS

 \Box Bulk density – 2.08 Mg/m³

Cohesion - 65 KN/m²

 \Box Angle of internal friction- 9 0

% fines – 8 % – 25 %

% Sand- 66% -89%

% Gravel - 1% - 13%

Soil classification: SC

Plasticity index- 21% - 22%

Liquid limit: -35% - 38%

Compressibility: medium

					SI	J MM A	ARY OF	ELABC	RATO	RY TE	EST RESU	JLTSC	F LASU	TH, LAG	OS STATI	Ε				
BH/	T	Depth	Particle	e Size D	istributi	on	Nat.	Δ	atterberg		B	SG		Quick Und	rained					
Sample			% Passing:		assing:		M.C	C Limits		Limits				Triaxial Te			Soil Class (UCS)	OedometerCons		
No.	P E	(m)	2.00 Mm	425 □m	63 □m	2 □m	%	LL % Dw	PL % dw	PI % D w	Mg/m3		Dia. Type (mm)	3 KN/m2	Cu KN/m2	Ø Deg.		Stress Range KN/m ²	Mv m2/M N	Cv m2/yr
DYLOI	\dashv																			
BH 01	D	0.75	99	91	10		12	35	14	21										
	$\frac{D}{U}$	1.00	98	90	25		13	38	17	21	2.08		38	100	65	9		50-100	0.154	2.8
	$\frac{c}{D}$	2.25	91	89	20		13				2.00		30	200	05			100-200	0.159	2.9
	D	3.00	90	86	24									400				200-400	0.143	3.0
												Ì						400-800	0.145	3.0
	D	5.75	89	88	23		12	37	15	22										
		7.50	87	85	8		14													
	_																			
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7.0 DEDUCTIONS AND RECOMMENDATIONS

Generally, the sub-soil stratification at this site is as shown by the geotechnical borehole drilled on the project site.

The lithology consists essentially of Dark brown gravely silty SAND, Dark Brown CLAYEY SAND, Brownish firm SANDY CLAY, Brownish stiff SANDY CLAY with occasional gravel, and Reddish Brown Ferrugenized Lateritic SANDY CLAY with Hardpan inclusions. Groundwater was not encountered in the drilled geotechnical borehole.

Soil characteristics

The sub-soil at this location is classified as INORGANIC CLAYS (CH) of high plasticity and SC based on the Unified Soil Classification System (UCS). The Atterberg's Limits values are moderate with plasticity Index between 21% to 22%, and liquid limits values ranged between 35 % to 38 %. From the Atterberg limits results it can be deduced that the soil is of medium compressibility. Gravel content is between 1 % to 13 %, while the sand content varies between 66 % and 89 %. The fines (silt and Clay) content varied between 8% and 25%.

Groundwater

Groundwater was encountered at surface level in the geotechnical boreholes drilled during this investigation which could be sampled or tested. However, as a result of the appreciable number of fines in the subsoil, hence there can be a softening of the soil with increase in moisture content.

Design/Allowable bearing Pressure

In other to arrive at the bearing pressure values for the design of the foundation of the proposed structural development at the project site, the allowable bearing pressure and settlement estimates were carried out and used to compute for various footing sizes (1.00 m, 1.50 m, 2.00 m, 3.00 m), using the data generated from the field (In-situ) tests and the laboratory oedometer consolidation settlement tests.

The following *design allowable bearing pressure (ABP)* values and settlement were computed for the sub-soil at this site. The result of the Computations is presented in table 8 below.

The settlement of the soil is within the range of 4.77 mm to 90.63 mm.

TABLE 8: ALLOWABLE BEARING PRESSURE (ABP) AND SETTLEMENT ESTIMATES FOR SHALLOW FOUNDATION AT THE PROJECT SITE

M_{v}	σ	B (m) = 1.5		B (m) = 2.25		B (m) = 2		B (m) = 3	
(m2/MN)	(KN/m2)	H (m)	S (mm)	H (m)	S (mm)	H (m)	S (mm)	H (m)	S (mm)
	30	1	4.77	1.5	7.155	2	9.54	3	14.31
	40	1	6.36	1.5	9.54	2	12.72	3	19.08
	50	1	7.95	1.5	11.925	2	15.9	3	23.85
	60	1	9.54	1.5	14.31	2	19.08	3	28.62
	70	1	11.13	1.5	16.695	2	22.26	3	33.39
	80	1	12.72	1.5	19.08	2	25.44	3	38.16
	90	1	14.31	1.5	21.465	2	28.62	3	42.93
0.159	100	1	15.9	1.5	23.85	2	31.8	3	47.7
	110	1	17.49	1.5	26.235	2	34.98	3	52.47
	120	1	19.08	1.5	28.62	2	38.16	3	57.24
	130	1	20.67	1.5	31.005	2	41.34	3	62.01
	140	1	22.26	1.5	33.39	2	44.52	3	66.78
	150	1	23.85	1.5	35.775	2	47.7	3	71.55
	170	1	27.03	1.5	40.545	2	54.06	3	81.09
	180	1	28.62	1.5	42.93	2	57.24	3	85.86
	190	1	30.21	1.5	45.315	2	60.42	3	90.63

PAD FOUNDATION DESIGN

DEPTH (meters)	FOOTING DIMENSIONS (L X B)	ABP (KN/m) ²	SETTLEMENT (mm)
	(meters)		
1.50	2.0 X 1.5	120	28.62 31.01
	3.0 X 2.0		34.98 38.16

The settlement of the sub-soil ranged from 5.13mm to 97.47mm, hence suitable for SHALLOW FOUNDATION as it is within the maximum tolerable settlement limits (\leq 40mm in SANDY soils and \leq 65mm on CLAYEY soils).

Consequently, an allowable bearing pressure (ABP) value of 120 KN/mmay be adopted for the design of the PAD foundation of the proposed structural development at this site at depth of -1.50 meters, provided it satisfies the Structural Engineers requirements for the proposed building.

It is advisable that the pads be tied together to mitigate settlement and ensure better rigidity of the structure.

Foundation Construction Works

Foundation excavation trench opened at this site during construction must be completely backfilled and compacted quickly to protect it from moisture and water on completion of foundation works. Topographically, the project site is on a low-lying terrain.

For concrete works, we recommend the use of rich, dense mix of ordinary Portland cement to satisfy the standard equivalent to CLASS 2 in the Code of Practice for foundations B.S. 8004: 1984, together with good embedment of reinforcement in all concrete structures at this project site.

The observations and deductions made in this report are based on the ground condition as revealed by the position of the geotechnical borehole and soil sampling which were carried out at the project site as well as visual site observation and laboratory tests on the recovered disturbed and undisturbed soil samples.

The structural/foundation design Engineer for the project shall appraise the prevailing sub-soil conditions as presented herein along with the proposed structures foundation option to arrive at the best choice of foundation type for the proposed development at the site.

PREPARED BY:

ADEYEMI IFEOLUWA O (MNAEGE, MNMGS)



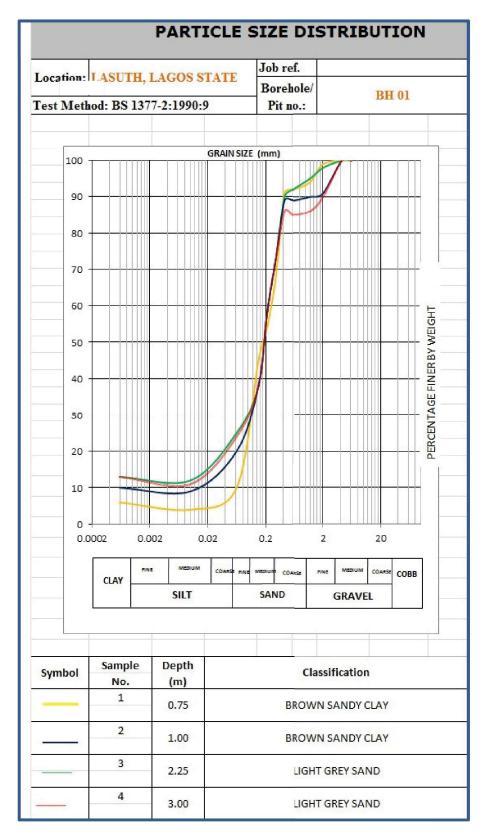


Fig 1.0: GEOTECHNICAL INVESTIGATION BEING CARRIED OUT ON SITE

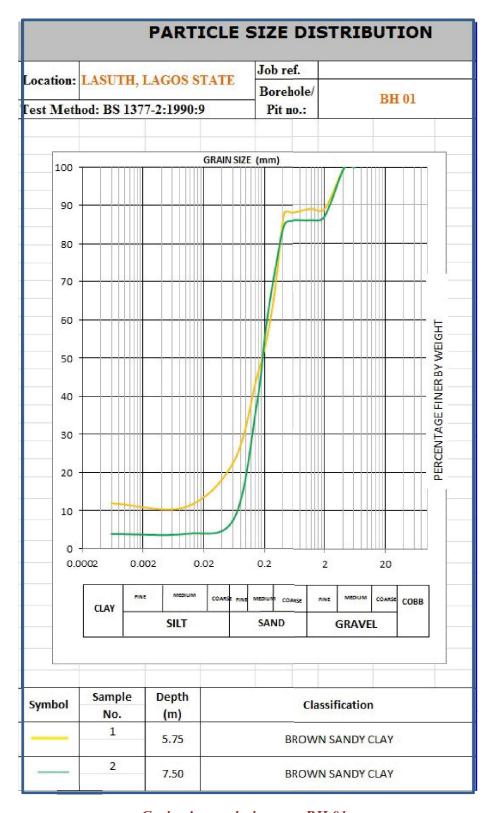


Fig 2.0: GEOTECHNICAL INVESTIGATION BEING CARRIED OUT ON SITE

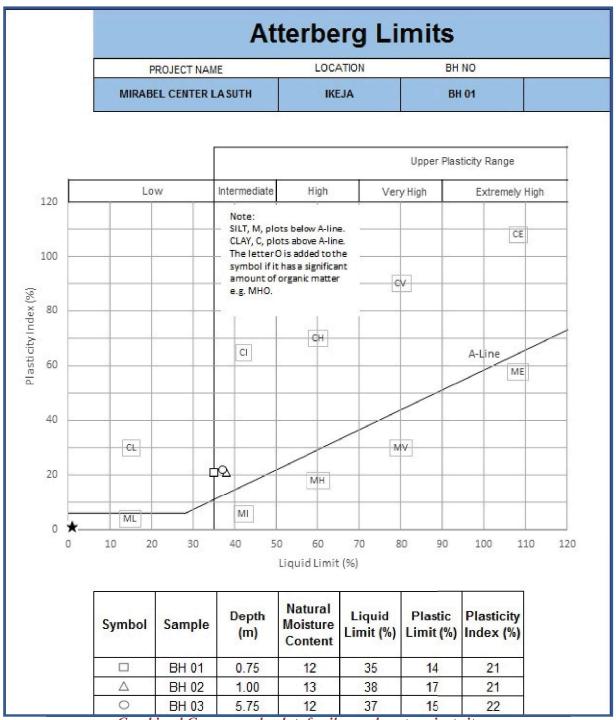
APPENDIX



Grain size analysis curve BH 01



Grain size analysis curve BH 01



Combined Cassagrande plotof soil samples at project site

LABORATORY QUICK UNDRAINED COMPRESSION TEST

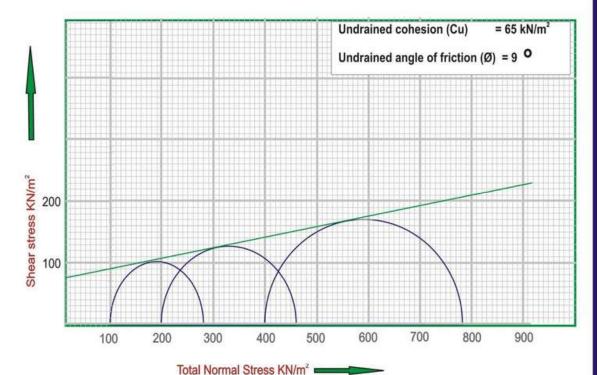
MOHR'S CIRCLE DIAGRAM

PROJECT: MIRABEL CENTER, LASUTH

BH NO ..: BH 01

SAMPLE NO: 3 SAMPLE TYPE: Undisturbed Sample

DEPTH: -1.00 m DATE: SEPTEMEBR. 2025



Cell Pressure σ_3 (KN/m²)	Max. Deviation Stress σ_1 - σ_3 (KN/m^2)	Total Normal Stress (KN/m²) σ₁
100	170	280
200	260	460
400	380	780

ANNEX 6A-CONTAINER SELECTION CHECKLIST

Container Selection Checklist:

This checklist shall be used by QA/QC inspectors and procurement staff when selecting, modifying, and approving containers for use in the Mirabel SARC project.

☐ Confirm container type: 40-ft High Cube (preferred) or 40-ft GP (only if required).
☐ Verify material: Corten steel with mill certificates provided.
\Box Check CSC plate: serial number, stacking rating, and max gross weight (valid and legible).
\square Inspect physical condition: no corrosion pitting/perforations, severe dents, twist, or racking.
☐ Corner castings intact: bolt holes round, not ovalized; plumb and square verified.
\square Structural soundness: corner posts, rails, and cross-members undeformed and load-bearing.
\square Welds on modifications: executed per approved WPS/PQR, with NDT reports (VT, PT/MT).
☐ Surface preparation: abrasive blast Sa 2½, coating DFT measurements recorded.
☐ Protective coating: zinc-rich primer, epoxy intermediate, PU topcoat, cavity wax in enclosed areas.
\square Insulation suitability: sufficient internal headroom for rockwool/PIR insulation and linings.
\square Weighbridge ticket provided post-modification (target 7.5–9.5 t per module).
☐ Lifting lugs and handling gear certified; lifting plan approved.
☐ Documentation complete: inspection reports, welder certificates, coating logs, weighbridge records

ANNEX 6B-INSTRUCTION TO BIDDERS

INSTRUCTION TO BIDDERS (ITB)

Project: Mirabel Sexual Assault Referral Centre (SARC), LASUTH, Lagos

Client: International IDEA (RoLAC 2 Programme)

Consultant: Olu Tee Engineering International Ltd

Document Purpose: Practical instructions to bidders consistent with ToR; to be read together with the tender notice/RFP, Drawings, BoQ, and QA/ITP.

1. SCOPE OF WORK

- 1.1 The Contractor shall construct, fit-out, and commission the container-based Mirabel SARC facility strictly in accordance with the Issued-for-Tender (IFT) drawings, BoQ, and these Instructions.
- 1.2 Works include civil/foundations, off-site container preparation, on-site assembly, architectural finishes, MEP (electrical, mechanical HVAC split units, plumbing/drainage), and testing & commissioning.
- 1.3 The Contractor shall coordinate with the Consultant for all submittals, inspections, and hold/witness points as detailed in the QA/QC ITP.

2. BIDDER ELIGIBILITY & QUALIFICATIONS

- 2.1 The Bidder shall demonstrate experience delivering at least two (2) modular/containerized or steel-frame building projects of similar scale within the last five (5) years.
- 2.2 The Bidder shall provide current registrations, tax compliance, and relevant professional licenses (e.g., COREN/ARCON-affiliated personnel where applicable).
- 2.3 The Bidder shall identify a Project Manager, Site Engineer, QS, and HSE Officer with CVs and certifications.

3. SITE VISIT & DUE DILIGENCE

- 3.1 The Bidder shall attend the pre-bid site visit (when scheduled) and is responsible for examining site conditions, access constraints, and logistics.
- 3.2 The Bidder shall allow for all temporary work and protection of existing hospital operations.

4. TECHNICAL REQUIREMENTS

- 4.1 Containers: The Contractor shall use ISO-certified 40-ft High Cube (HC) Corten steel containers in sound structural condition with valid CSC plates. No perforations/pitting; no severe dents/twist/racking. Corner castings shall be intact with round, non-ovalized bolt holes; interfaces shall be plumb and square (vertical ±5 mm; diagonal squareness within 3 mm at 2 m).
- 4.2 Structural Integrity: Corner posts, rails, and cross-members shall remain loadbearing after modifications. All structural works shall comply with the Structural Engineer of Record (SER) calculations and the final IFC drawings.
- 4.3 Welding & Fabrication: Execute welding per approved WPS/PQR. Perform NDT (VT and PT/MT as applicable). Submit weld maps and NDT reports in the QA dossier.
- 4.4 Surface Prep & Coatings: Blast-clean to Sa 2½ (ISO 8501-1). Record Dry Film Thickness (DFT) for each coat. Protective system: zinc-rich primer + epoxy intermediate + polyurethane topcoat. Apply cavity wax to enclosed voids after services integration.
- 4.5 Insulation & Interiors: Verify headroom prior to procurement. Provide rockwool (≥75 mm) or PIR insulation to meet thermal/acoustic requirements, coordinate with linings and services.
- 4.6 HVAC (Split Units): Provide and install split-unit AC per schedule and drawings. At commissioning, verify a 10–12°C supply-air temperature reduction within 20 minutes of start-up for each indoor unit.
- 4.7 Plumbing & Drainage: Provide complete cold/hot water distribution, sanitary drainage, and fixtures. Plumbing scope is separate from AC but coordinated under MEP.
- 4.8 Electrical: Install per drawings and schedules, including earthing $\leq 0.4 \Omega$, lighting (LED), small power, and dedicated AC circuits. Provide PV-ready conduits and space for inverter/batteries as shown in drawings.
- 4.9 Rainwater Harvesting (RWH): Provide gutters/downpipes to storage tank with first-flush diverter, leaf guards, and coarse filtration (200–500 μ m) pre-tank. Include pump and disinfection point for non-potable reuse (cleaning/irrigation).
- 4.10 Sustainability: Fit LED lighting, low-flow fixtures, adequate insulation/ventilated façade details, and cable pathways for future PV integration as indicated.
- 4.11 Lifting/Handling & Weight: Provide certified lifting lugs. Submit weighbridge tickets for each container module (target post-mod mass typically 7.5–9.5 t).

5. DRAWINGS, APPROVALS, AND SEQUENCING

- 5.1 Build strictly to the latest IFT/IFC drawings and the BoQ. No deviation shall occur without written instruction from the Consultant.
- 5.2 Statutory approvals (LASPPPA, LASBCA, LASEPA, Fire Service, NCPWD, and others as applicable) shall be observed. Work that requires approvals shall not proceed until sign-offs are in place.

5.3 Off-site prefabrication may proceed in parallel with on-site foundation works and curing, subject to QA/ITP witness/hold points and approvals sequencing.

6. QA/QC, TESTING AND ITP HOLD POINTS

- 6.1 The Contractor shall comply with the Inspection & Test Plan (ITP) including hold/witness points for: concrete slump and cube tests; weld NDT; coating DFT; earthing resistance; plumbing hydrostatic tests; electrical functional tests; and HVAC commissioning.
- 6.2 No work shall proceed beyond a hold point until the Consultant has reviewed and accepted relevant test results and inspection records.
- 6.3 The Contractor shall submit a QA dossier including WPS/PQR, NDT reports, DFT logs, weighbridge tickets, material certificates, and commissioning sheets.

7. HEALTH, SAFETY & ENVIRONMENT (HSE)

- 7.1 The Contractor shall implement a HSE Plan compliant with Nigerian labor laws and local regulations, including LASEPA environmental controls, fire safety, and hospital access protocols.
- 7.2 Provide PPE, method statements, risk assessments (JSA), and emergency procedures. Maintain clean site logistics within the hospital precinct.

8. SUBMITTALS

- 8.1 The Contractor shall submit the following prior to procurement/installation: shop drawings, product data sheets, method statements, WPS/PQR, ITP/QC checklists, samples/mockups (where requested).
- 8.2 As-built drawings and O&M manuals shall be submitted at handover, including warranties and commissioning certificates.

9. PROGRAMME AND MILESTONES

- 9.1 The Contractor shall submit a detailed programme (Gantt) reflecting a four-month delivery with parallel off-site fabrication and on-site works.
- 9.2 Dates/timelines shall be re-aligned to the award date as applicable. Any slippage and recovery plans shall be notified within five (5) working days.

10. BOQ & PRICING INSTRUCTIONS

- 10.1 Prices shall be based on the provided BoQ (NIQS format). Where discrepancies arise between drawings and BoQ descriptions, the more stringent requirement shall govern, and the Bidder shall notify queries before bidding close.
- 10.2 The Edited BoQ and the cost plan summaries form the pricing baseline for evaluation. All rates shall be deemed to include preliminaries, overheads, and profit unless otherwise stated.
- 10.3 Provisional sums (if any) and prime cost items shall be clearly identified.

11. BID FORMAT & SUBMISSION

- 11.1 Submit a single PDF (technical + financial) and an editable BoQ (XLSX) as instructed in the RFP.
- 11.2 Technical: methodology, programme, QA/QC approach, HSE plan, team CVs, relevant experience, approvals strategy, and shop drawings list.
- 11.3 Financial: completed BoQ, grand summary, allowances, and list of exclusions (if any).
- 11.4 Validity: Bid validity and submission deadline shall follow the RFP. Late submissions will not be accepted.

12. CLARIFICATIONS AND ADDENDA

- 12.1 Bidders shall submit clarification questions within the timeframe stated in the RFP. Responses and any addenda issued shall form part of the ITB and be acknowledged in the bid.
- 12.2 No verbal instructions are binding unless confirmed in writing.

13. EVALUATION REFERENCE

13.1 Evaluation shall follow the RFP criteria and scoring matrix issued by the Client. Compliance with these Instructions, QA/ITP alignment, and completeness of the BoQ will be key determinants.

This Instruction to Bidders must be read together with ToR, the tender notice/RFP, the Drawings (IFT/IFC), the BoQ, the QA Management Plan, and the QA/QC ITP Pack.

ANNEX 6CMIRABEL SARC GLOSSARY OF ABBREVIATIONS

Mirabel SARC — Glossary of Abbreviations (A-Z)

Alphabetical list of abbreviations used across ToR, RFP, ITB, drawings, QA/QC plans, HSE plan, BoQ, and minutes.

Abbreviation	Full Term	Definition / Project Context	Key Source Docs
AC	Air Conditioner	Split-unit cooling equipment per schedule and drawings.	Design Report; ITP
ARCON	Architects Registration Council of Nigeria	Professional council; architects' seals and compliance.	Regulatory Constraints
BoQ / BOQ	Bill of Quantities	Itemized schedule of works, quantities, and rates (NIQS format).	BoQ Files; RFP
ВРР	Bureau of Public Procurement	Procurement oversight body (referenced where applicable).	Procurement Context
COC	Certificate of Conformity	Document certifying compliances of materials/equipment.	QA Dossier
COREN	Council for the Regulation of Engineering in Nigeria	Engineering practice regulator; engineers' seals and compliance.	Regulatory Constraints
CSC	Convention for Safe Containers	International convention; containers must carry a valid CSC plate.	Container Checklist
DFMA	Design for Manufacture and Assembly	Approach emphasizing off-site fabrication and modular assembly.	Methodology/Programme

DFT	Dry Film Thickness	Measured thickness of protective coatings; recorded per coat.	ITP; Coatings
G+3	Ground plus Three Floors	Stacking reference used in preliminary corner-reaction notes.	Design Report Annex B
НС	High Cube (Container)	40-ft high-cube ISO container type specified for modules.	Design Report; Container Checklist
HSE	Health, Safety and Environment	Site safety and environmental management requirements.	HSE Plan; ToR; ITB
HVAC	Heating, Ventilation and Air Conditioning	Mechanical systems for thermal comfort and air quality (split units).	Design Report; ITP
IFC	Issued for Construction	Final, signed drawing set for construction.	Drawings; ITB
IFT	Issued for Tender	Drawing/spec set issued for bidding purposes.	Drawings; RFP
ISO	International Organization for Standardization	Reference standards (e.g., ISO 8501-1 for surface preparation).	Coatings; QA
ITF	Industrial Training Fund	Training contributions and compliance (where applicable).	Corporate Compliance
ITP	Inspection and Test Plan	Matrix of hold/witness points, inspections, and acceptance criteria.	QA/QC ITP Pack

JSA	Job Safety Analysis	Task-based risk assessment required before high-risk works.	HSE Plan
LASBCA	Lagos State Building Control Agency	State agency for building control, stage approvals and supervision.	Regulatory Constraints
LASEPA	Lagos State Environmental Protection Agency	Regulator for environmental standards and site HSE/EM safeguards.	Regulatory Constraints; HSE
LASPPPA	Lagos State Physical Planning Permit Authority	State authority for planning permits and development approvals.	Regulatory Constraints
LASUTH	Lagos State University Teaching Hospital	Project host institution and site for Mirabel SARC.	Approvals; Meeting Minutes
LED	Light-Emitting Diode	Energy-efficient lighting type required in sustainability section.	Design Report
LSSC	Lagos State Safety Commission	State regulator for safety practices and site audits.	Regulatory Constraints; HSE
MEP	Mechanical, Electrical and Plumbing	Combined services package covering HVAC, power/lighting, water/drainage.	Design Report; ELECT DESIGN
MT	Magnetic Particle Testing	NDT method for detecting surface/near-surface discontinuities in ferromagnetic materials.	ITP; Fabrication

NCPWD	National Commission for Persons with Disabilities	Federal body for accessibility compliance (ramps, lifts, signage).	Regulatory Constraints; Design
NDT	Non-Destructive Testing	Testing methods that do not damage the part (e.g., VT, PT, MT).	ITP; Fabrication
NIA	Nigerian Institute of Architects	Professional association referenced for architect sign-offs.	Regulatory Constraints
NIQS	Nigerian Institute of Quantity Surveyors	Professional body; NIQS format adopted for BoQ.	BoQ Files
NSITF	Nigeria Social Insurance Trust Fund	HSE/social insurance compliance for contractors.	Corporate Compliance
O&M	Operations and Maintenance	Manuals and instructions required at handover.	Handover Requirements
PIR	Polyisocyanurate	Thermal insulation option; alternative to rockwool.	Design Report
PPE	Personal Protective Equipment	Mandatory safety gear for all site personnel.	HSE Plan
PQR	Procedure Qualification Record	Records demonstrating the WPS produces compliant welds.	ITP; Fabrication
PT	Penetrant Testing	Liquid penetrant NDT for surface-breaking defects in non-ferrous/ferrous metals.	ITP; Fabrication

PU	Polyurethane	Topcoat finish in the protective coating system.	Coatings Spec
PV	Photovoltaic	Solar power readiness: conduits, inverter space, cable pathways.	Design Report (Sustainability)
QA	Quality Assurance	Planned and systematic activities to ensure quality requirements are met.	QA Management Plan
QC	Quality Control	Operational techniques to meet quality requirements (inspections/tests).	QA Management Plan; ITP
RAMS	Risk Assessment and Method Statement	Safety and quality planning documents for critical activities.	HSE; QA/ITP
RFP	Request for Proposals	Bidding document stating submission rules and evaluation criteria.	RFP drafts
RoLAC	Rule of Law and Anti-Corruption Programme	EU/International IDEA programme funding and overseeing this assignment.	RFP/ToR; Cover Memos
RWH	Rainwater Harvesting	Gutters, downpipes, first-flush, filtration, storage for non-potable reuse.	Design Report (Sustainability)
SARC	Sexual Assault Referral Centre	Specialized clinical and psychosocial support centre; Mirabel SARC is within LASUTH.	Needs Assessment; Site/Design Reports

SER	Structural Engineer of Record	Engineer responsible for structural design and sign-off.	Design/Calculations
ToR	Terms of Reference	Scope, objectives, and deliverables of the consultancy/works.	ToR drafts
VT	Visual Testing	Visual examination of welds/surfaces for defects.	ITP; Fabrication
WPS	Welding Procedure Specification	Approved method for performing welds on structural works.	ITP; Fabrication

ANNEX 7CONSTRUCTIO NTIMELINE



KM 35, Lekki-Epe Expressway, Ibeju-Lekki, Lagos.
 info@olutee-ng.com
 01-2932794
 www.olutee-ng.com

Activity	Start Date	Finish Date	Duration (weeks)
Mobilization & Site Setup	2025-12-0	01 2025-12-08	3 1
Site Clearance & Preliminary Works	2025-12-0	08 2025-12-15	5 1
Geotechnical Confirmation & Surveys	2025-12-	15 2025-12-22	2 1
Foundation Works (excavation, RC pads, be	ea 2025-12-	22 2026-01-19	9 4
Offsite Container Prefabrication	2025-12-	22 2026-02-02	2 6
Container Delivery & Onsite Installation	2026-02-0	2026-02-23	3
Structural Reinforcements & Exoskeleton Ti	ie 2026-02-0	2026-02-16	5 2
Roofing & Weatherproofing	2026-02-	16 2026-03-02	2 2
MEP Rough-ins (plumbing, electrical, AC pip	oi 2026-03-0	2026-03-16	5 2
External Cladding & Insulation	2026-03-	16 2026-03-30) 2
Internal Partitions & Finishes	2026-03-	16 2026-03-30) 2
Final MEP Installations	2026-03-	16 2026-03-30) 2
Fire Safety & Accessibility Installations	2026-03-3	30 2026-04-06	5 1
Testing, Commissioning & QA/QC	2026-04-0	06 2026-04-07	1
Regulatory Inspections & Approvals	2026-04-0	07 2026-04-07	1
Training & Handover	2026-04-0	07 2026-04-07	1